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property of the Architect and no part thereof shall be copied, disclosed to others or used in connection with any other work or project, except for the specified project for which they have been prepared and developed, without the prior written consent of the Architect. The Contractor shall verify, and be responsible for, all dimensions and conditions on the job and the Architect shall be notified of any variations from or discrepancies between the dimensions and conditions shown on this set of drawings.

OVERMYER

A R C H I T E C T S

3213 P STREET, N.W.

WASHINGTON, D.C. 20007

VOICE: (202) 333-5596

FACSIMILE: (202) 333-5598

The above drawings and specifications and all ideas

designs, inventions and arrangements represented

PERMIT AMENDMENT FOR

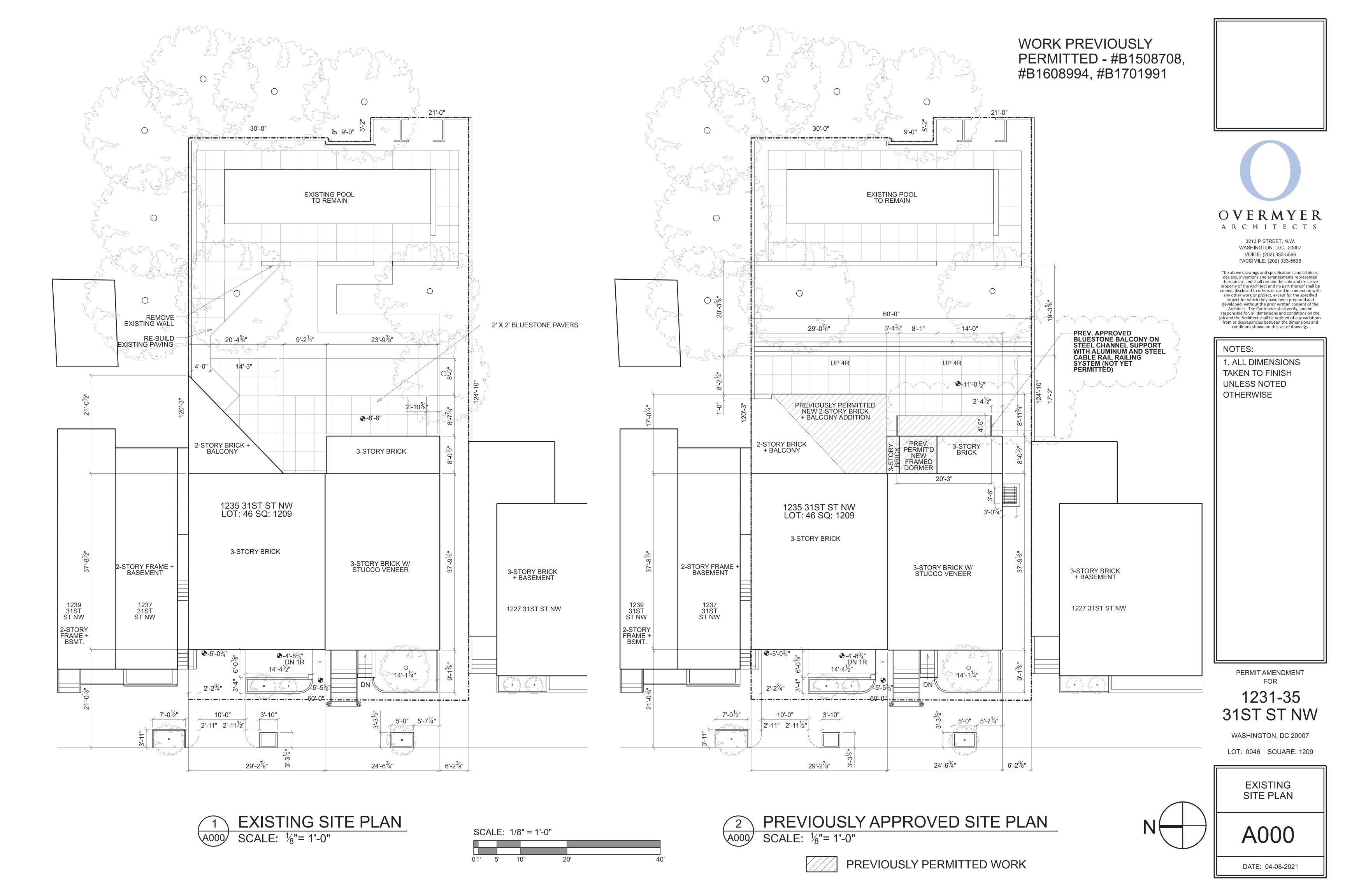
1231-35 31ST ST NW

WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

VICINITY PLAN / COVER SHEET

0001



ZONING DATA

LOT SQUARE FOOTAGE = 7306.04 S.F.

CURRENT HOUSE (BUILT 1973)
BUILDING FOOTPRINT = 2439.83 S.F.
LOT OCCUPANCY = 33.4%

PREVIOUSLY PERMITTED HOUSE BUILDING FOOTPRINT = 2719.83 S.F.. LOT OCCUPANCY = 37.2%

PERMIT AMENDMENT BUILDING FOOTPRINT = 2,873.63 S.F. (5.8% INCREASE FROM EXISTING) LOT OCCUPANCY = 39.3%

EXISTING GROSS SQUARE FOOTAGE = 7,105.06 S.F.

PREVIOUSLY PERMITTED GROSS SQUARE FOOTAGE = 8,649.26 S.F.

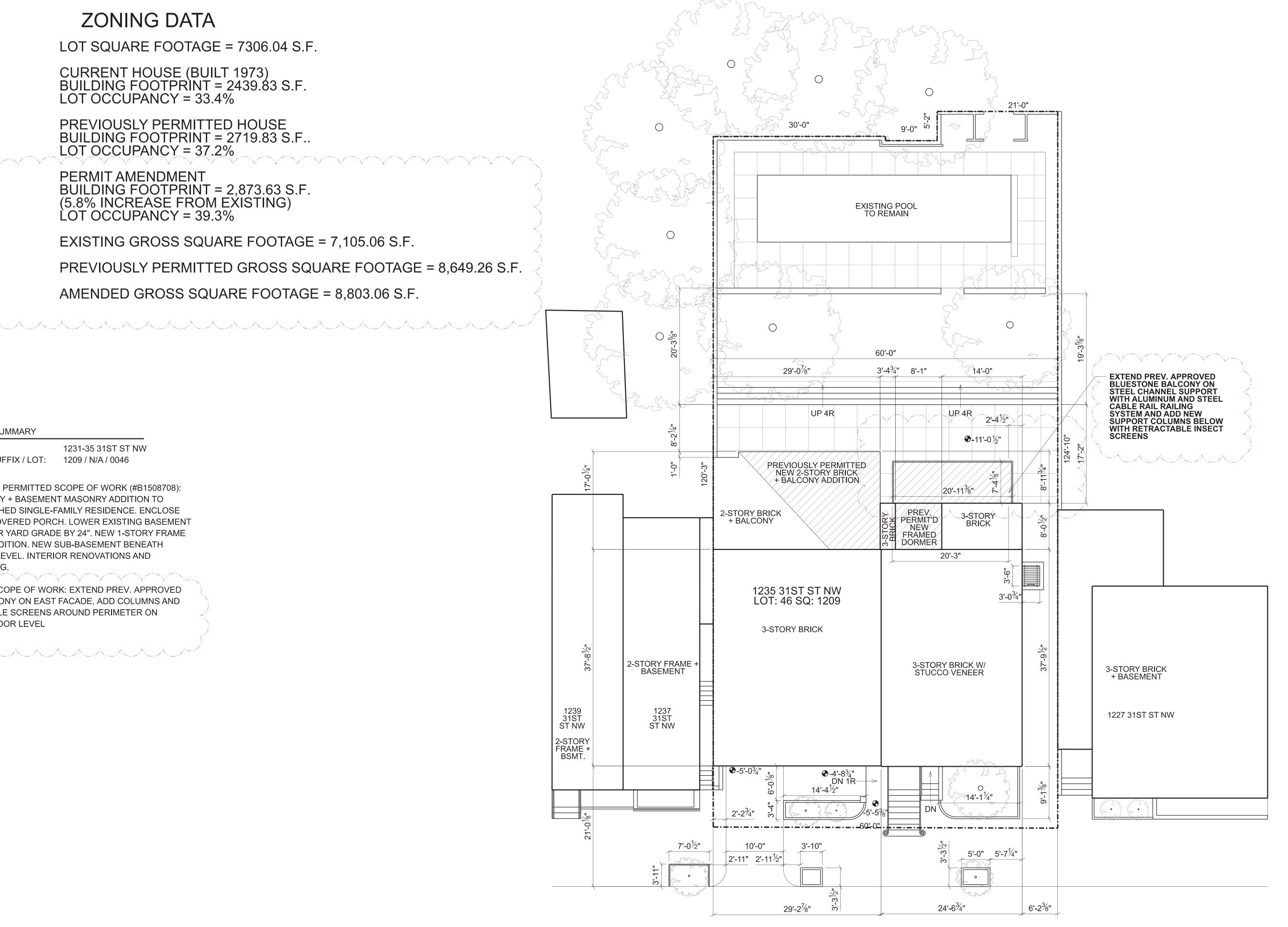
AMENDED GROSS SQUARE FOOTAGE = 8,803.06 S.F.

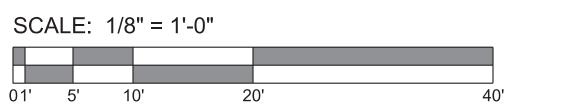
PROJECT SUMMARY

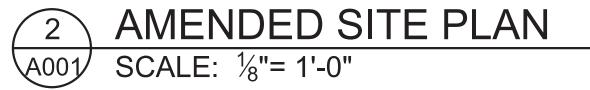
ADDRESS: 1231-35 31ST ST NW SQUARE / SUFFIX / LOT: 1209 / N/A / 0046

PREVIOUSLY PERMITTED SCOPE OF WORK (#B1508708): NEW 1-STORY + BASEMENT MASONRY ADDITION TO SEMI-DETACHED SINGLE-FAMILY RESIDENCE. ENCLOSE EXISTING COVERED PORCH. LOWER EXISTING BASEMENT LEVEL / REAR YARD GRADE BY 24". NEW 1-STORY FRAME DORMER ADDITION. NEW SUB-BASEMENT BENEATH BASEMENT LEVEL. INTERIOR RENOVATIONS AND RE-MODELING.

AMENDED SCOPE OF WORK: EXTEND PREV. APPROVED STEEL BALCONY ON EAST FACADE, ADD COLUMNS AND RETRACTABLE SCREENS AROUND PERIMETER ON GROUND FLOOR LEVEL









SITE PLAN A001

DATE: 04-08-2021

PERMIT AMENDMENT

FOR

1231-35

31ST ST NW

WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

AMENDED

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conditions shown on this set of drawings.

1. ALL DIMENSIONS

TAKEN TO FINISH

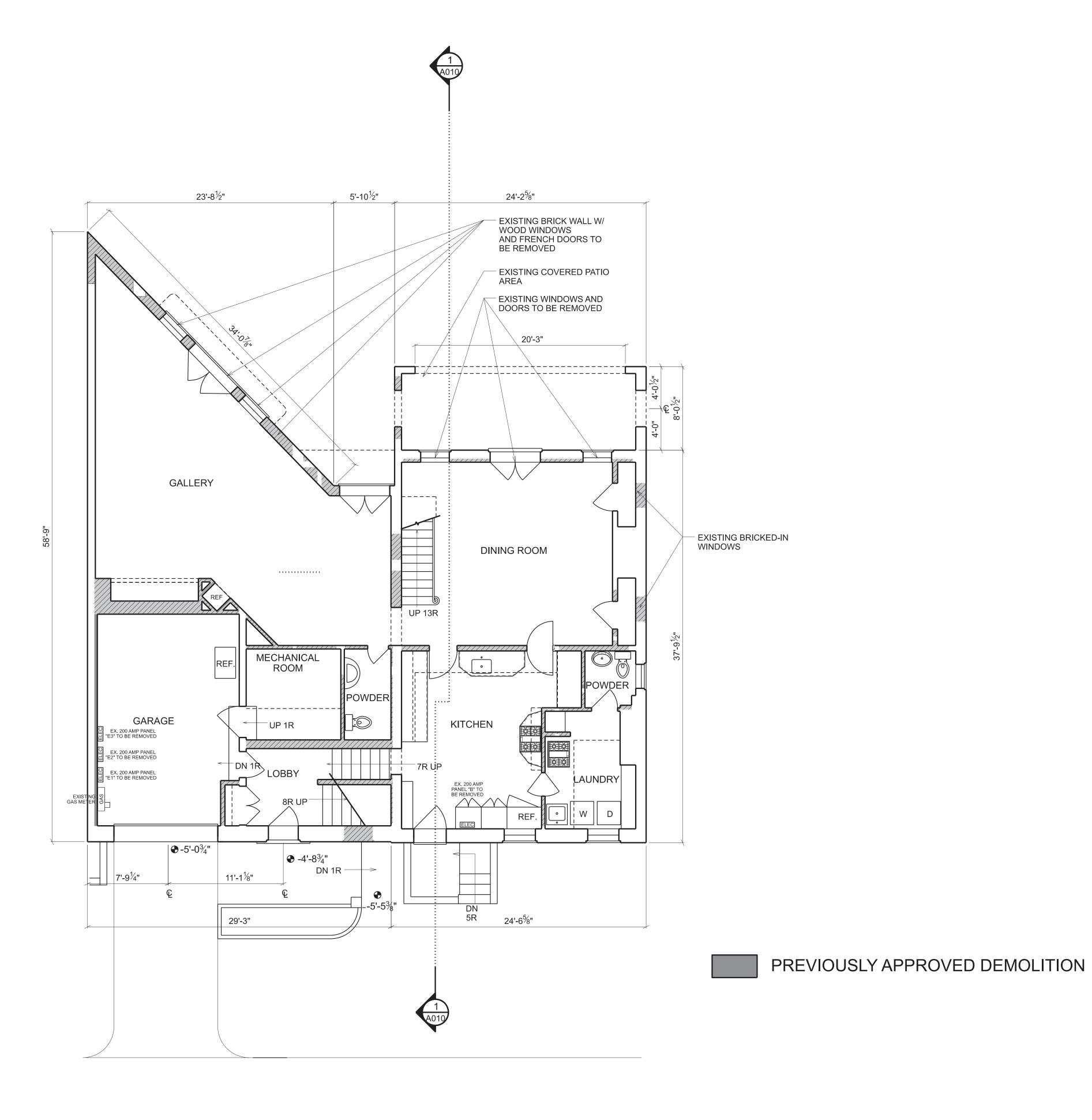
UNLESS NOTED

OTHERWISE

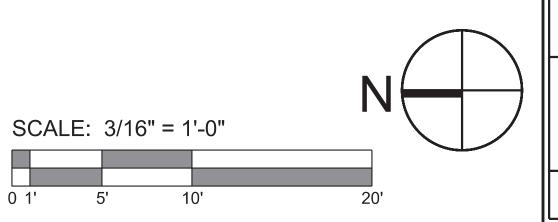
NOTES:

AREAS OF NEW CONSTRUCTION (PERMIT AMENDMENT)

WORK PREVIOUSLY PERMITTED - #B1508708. #B1608994, #B1701991



1 EXISTING BASEMENT FLOOR PLAN
A002 SCALE: 3/16"= 1'-0"



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ARCHITECTS

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NOTES:

1. ALL DIMENSIONS
TAKEN TO FRAMING
UNLESS NOTED
OTHERWISE

2. ALL WALLS TO BE 2X4 FRAME UNLESS NOTED OTHERWISE

> PERMIT AMENDMENT FOR

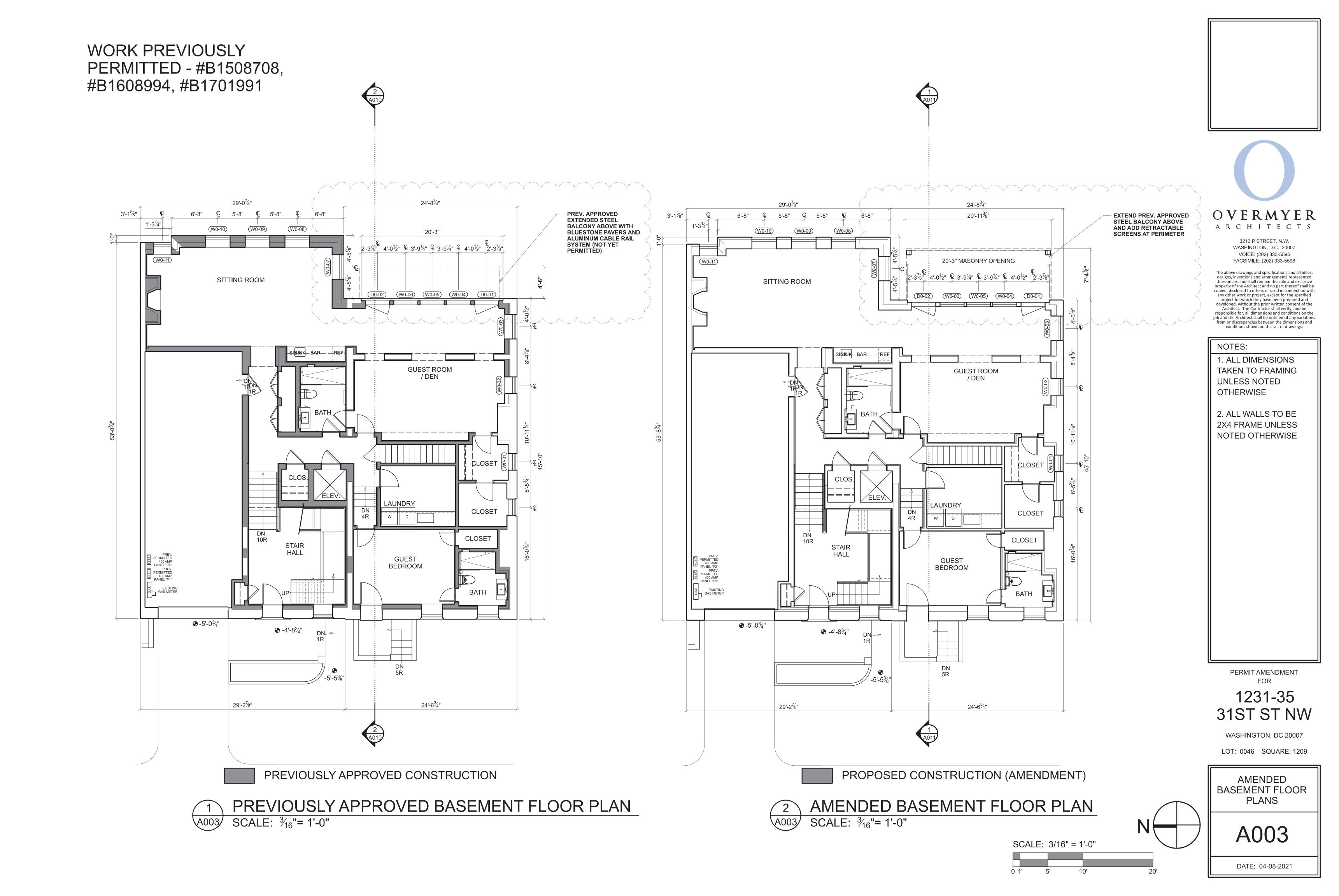
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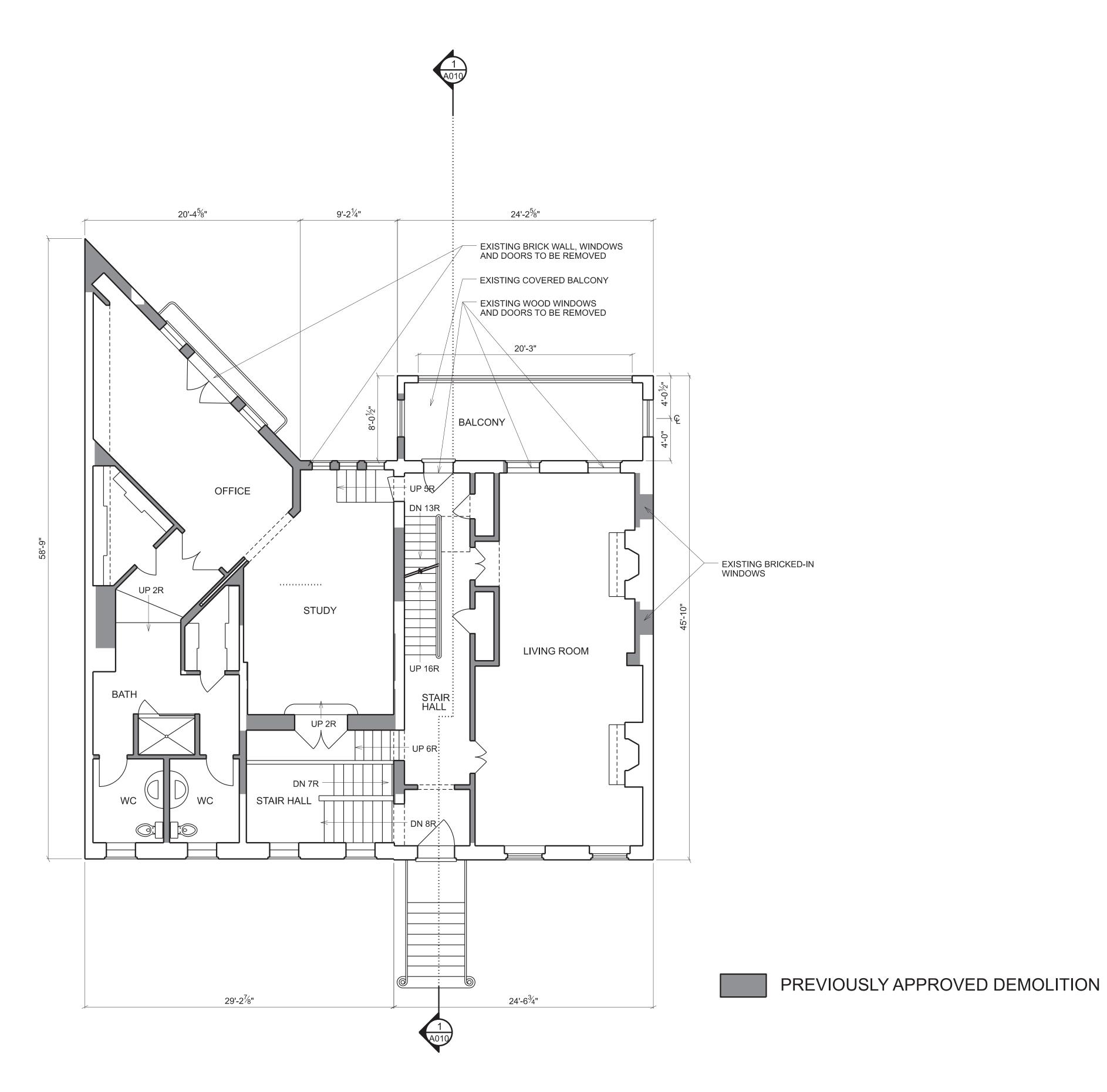
LOT: 0046 SQUARE: 1209

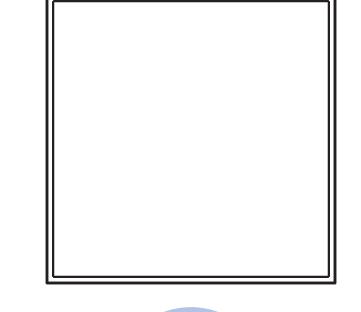
EXISTING BASEMENT FLOOR PLANS

A002



WORK PREVIOUSLY PERMITTED - #B1508708. #B1608994, #B1701991







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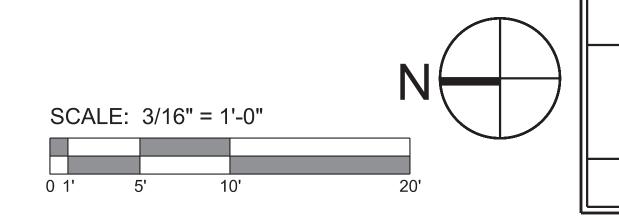
> PERMIT AMENDMENT FOR

1231-35 31ST ST NW

WASHINGTON, DC 20007

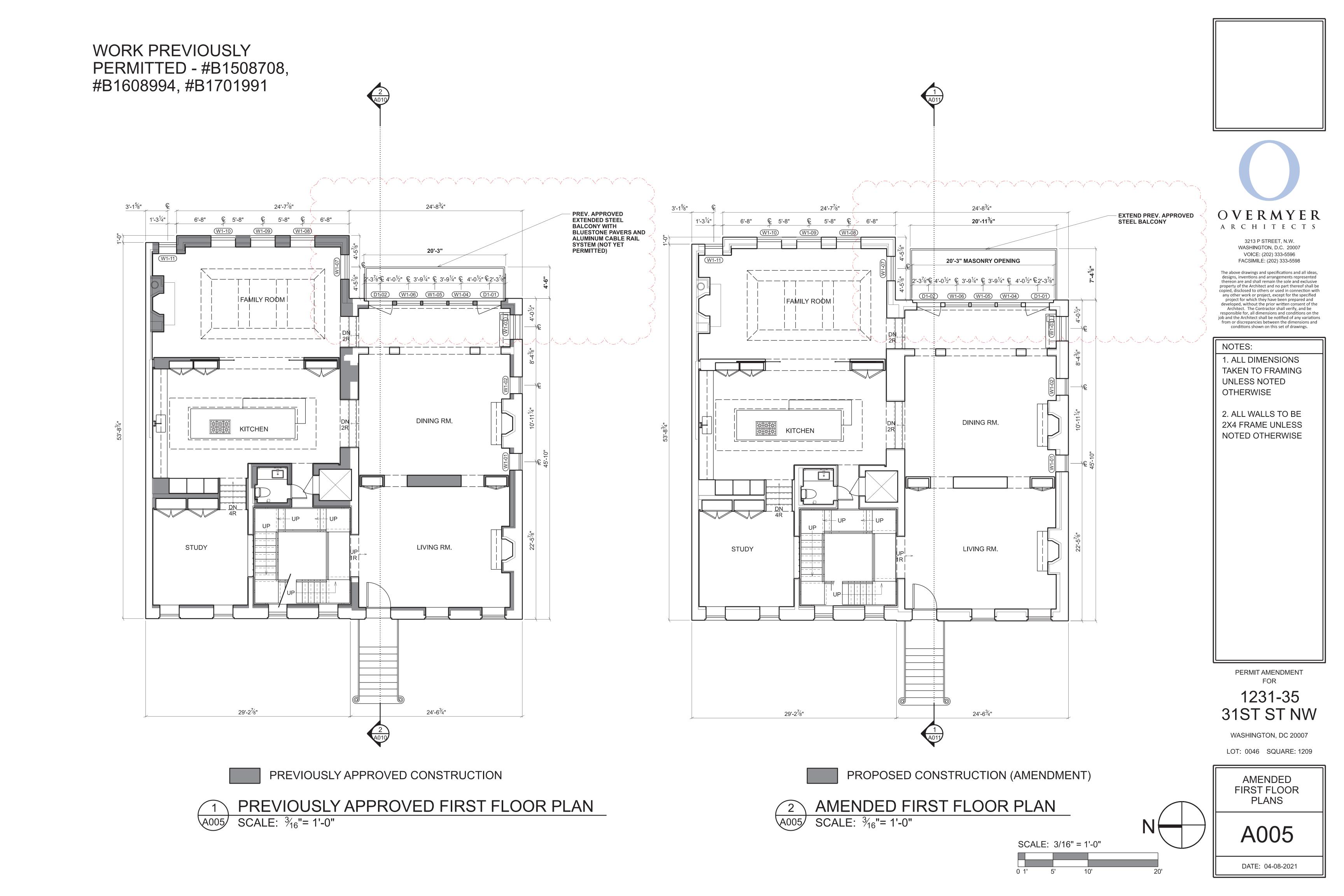
LOT: 0046 SQUARE: 1209





EXISTING FIRST FLOOR PLANS

A004

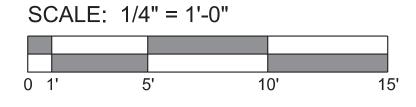


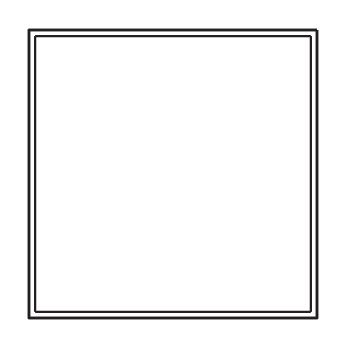


WORK PREVIOUSLY PERMITTED - #B1508708, #B1608994, #B1701991



AMENDED SOUTH EXTERIOR ELEVATION
SCALE: 1/4"= 1'-0"







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PERMIT AMENDMENT FOR

1231-35 31ST ST NW

WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

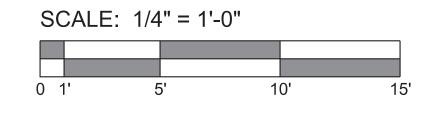
AMENDED SOUTH EXTERIOR ELEVATION

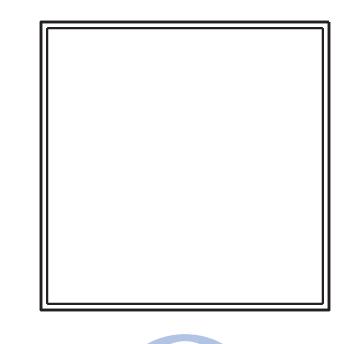
A007

WORK PREVIOUSLY PERMITTED - #B1508708, #B1608994, #B1701991



1 EXISTING EAST EXTERIOR ELEVATION
A008 SCALE: 1/4"= 1'-0"







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PERMIT AMENDMENT FOR

1231-35 31ST ST NW

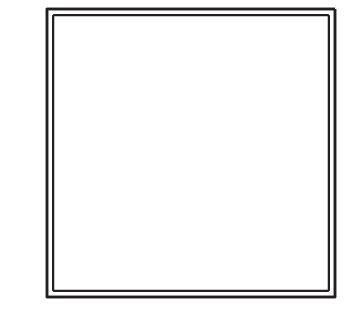
WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

EXISTING EAST EXTERIOR ELEVATIONS

800A







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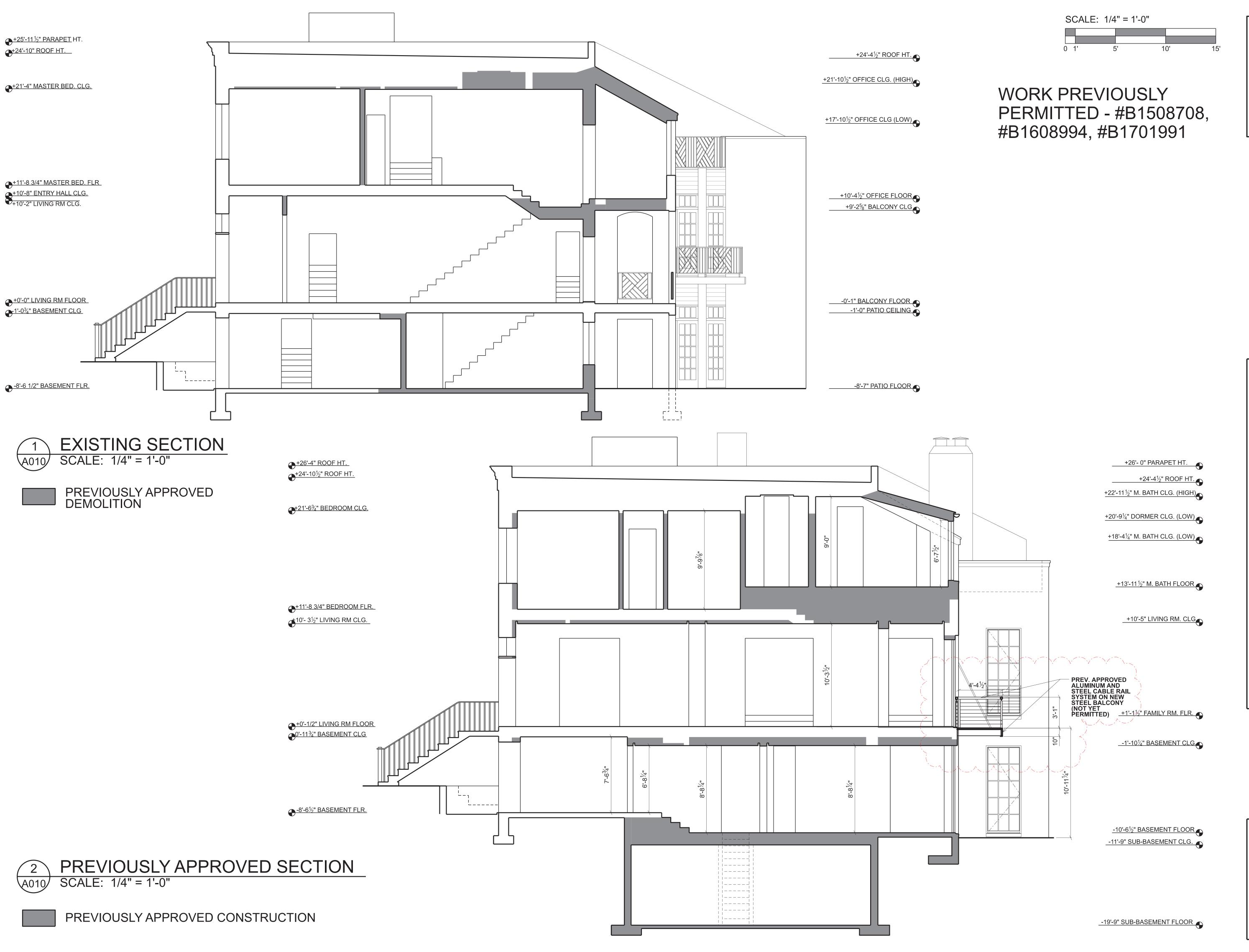
1231-35 31ST ST NW

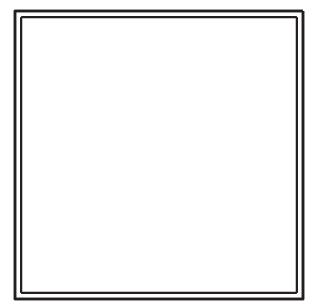
WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

AMENDED EAST EXTERIOR ELEVATIONS

A009





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OTHERWISE

PERMIT AMENDMENT

1231-35 31ST ST NW

WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

EXISTING SECTION

A010

WORK PREVIOUSLY PERMITTED - #B1508708, +26'-4" ROOF HT. +26'- 0" PARAPET HT. +24'-10½" ROOF HT. #B1608994, #B1701991 +22'-11½" M. BATH CLG. (HIGH) +21'-6³/₄" BEDROOM CLG. +20'-91/4" DORMER CLG. (LOW) +18'-41/4" M. BATH CLG. (LOW) +13'-11½" M. BATH FLOOR +11'-8 3/4" BEDROOM FLR. +10'-5" LIVING RM. CLG 10'- 3½" LIVING RM CLG. EXTEND PREV.
APPROVED
ALUMINUM AND
STEEL CABLE RAIL
SYSTEM ON NEW
STEEL BALCONY,
ADD NEW COLUMNS
BELOW WITH
RETRACTABLE SCREENS +1'-1½" FAMILY RM. FLR. +0'-1/2" LIVING RM FLOOR 0'-11¾" BASEMENT CLG -1'-101/4" BASEMENT CLG. -8'-61/2" BASEMENT FLR. -10'-61/2" BASEMENT FLOOR -11'-9" SUB-BASEMENT CLG. |---- $I - - - - - - \rightarrow$! i----i ! !----| | - - - - - | 2½" SQUARE ALUMINUM TOP CAP PAINTED BLACK ! !-----19'-9" SUB-BASEMENT FLOOR SCALE: 1/4" = 1'-0" ALUMINUM TOP CAP SADDLE, PAINTED BLACK PROPOSED SECTION PROPOSED CONSTRUCTION (AMENDMENT) SCALE: 1/4" = 1'-0" - ALUMINUM POST SADDLE, PAINTED BLACK FINISH - 1/4" DIA. STAINLESS STEEL CABLE, - 1.9" DIAMETER ALUMINUM POST, PAINTED BLACK FINISH, @ 4'-7" O.C. MAX. TYP. DETAIL THROUGH CABLE RAILING (PREVIOUSLY APPROVED) CABLE RAILING INSPIRATION IMAGE PAVER INSPIRATION IMAGE SCALE: 1 1/2" = 1'-0" NOT TO SCALE NOT TO SCALE

SCALE: 1-1/2" = 1'-0"

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OTHERWISE

PERMIT AMENDMENT FOR

1231-35 31ST ST NW

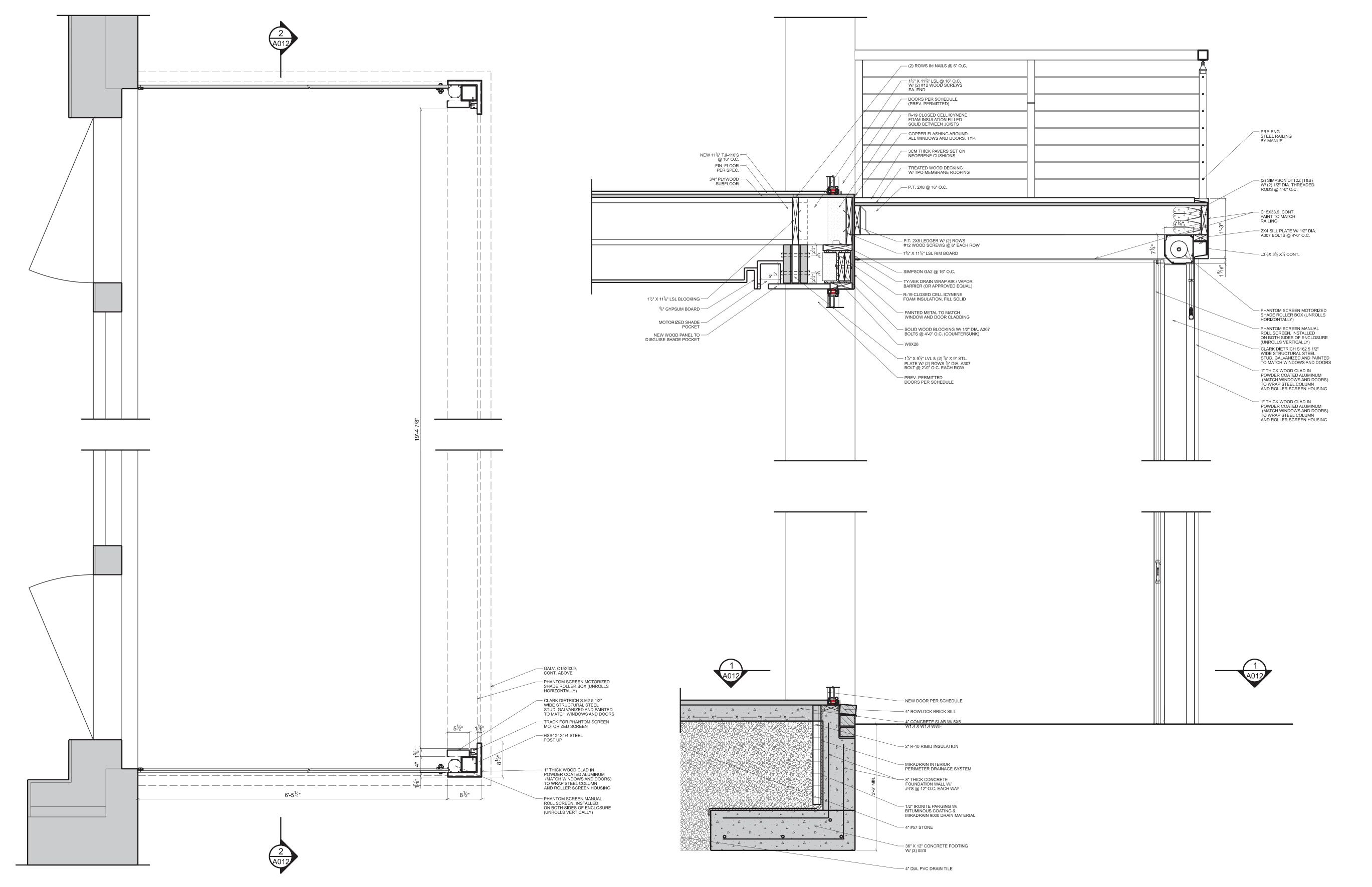
WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

PROPOSED SECTION

A011





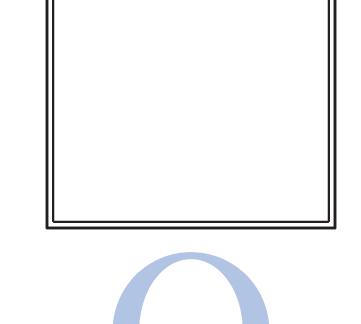
PLAN DETAIL THROUGH SCREENED PATIO

SCALE: 1" = 1'-0"

EXISTING / PREV. PERMITTED TO REMAIN



EXISTING / PREV. PERMITTED TO REMAIN



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1231-35 31ST ST NW

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LOT: 0046 SQUARE: 1209

SCREENED PATIO DETAILS

A012

DESIGN NOTES

DESIGN LIVE LOADS FOR NEW WORK

- A. ROOF LIVE LOAD
- Pg = 30 PSF, MIN ROOF DESIGN LOAD = 30 PSF
- 2. Pf = 21 PSF + DRIFTING
- 3. Ce = 1.0 4. Is = 1.0
- 5. Cs = 1.0
- B. FLOOR LIVE LOADS DWELLING AREAS
- 2. HANDRAILS = 50 PLF LATERAL OR 200 LBS POINT LOAD IN ANY DIRECTION

C. WIND LOAD

- V (3-second gust) = 90 MPH
- 2. **I**w= 1.0
- EXPOSURE = B
- D. SEISMIC LOAD
- LATERAL FORCE SYSTEM: BRACED WOOD PANELS SEISMIC USE GROUP = I
 - SITE CLASS = D
- NO DESIGN REQUIRED PER IRC/R301.2.2
- E. CODE: THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL RESIDENTIAL CODE 2012 AND THE 2013 DCMR.
- ASSUMED EARTH PRESSURES
- P AT REST = 60H
- P ACTIVE = 45H
- P PASSIVE = 300H G. DEAD LOADS
- WOOD FRAMED ROOF = 15 PSF
- TYPICAL WOOD FLOORS = 12 PSF
- CONCRETE DECK = 50 PSF
- 4. TILE/STONE FLOORS = 20 PSF
- TYPICAL ROOF = 15 PSF
- BALCONY W/ STONE PAVERS = 40 PSF

A. ALL JOISTS, BEAMS AND POSTS SHALL BE SPRUCE-PINE-FIR NO.1/NO.2 PER "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", NFPA. ALL STUDS SHALL BE SPRUCE-PINE-FIR STUD-GRADE. ALL WOOD MEMBERS SHALL BE MANUFACTURED TO COMPLY WITH PS20 OF "AMERICAN SOFTWOOD LUMBER STANDARDS" AND SHALL HAVE 19% MAXIMUM MOISTURE CONTENT.

- MINIMUM MEMBER PROPERTIES SHALL BE AS FOLLOWS:
- WOOD LINTELS, JOISTS AND BEAMS
- a) FLEXURE: Fb = 875 PSI
- b) SHEAR: Fv = 135 PSI
- c) MODULUS OF ELASTICITY = 1,400,000 PSI
- WALL STUDS: STUD GRADE
- a) FLEXURE: Fb = 675 PSI
- b) COMPRESSION PARALLEL: Fc" = 725 PSI
- c) MODULUS OF ELASTICITY = 1,200,000 PSI
- B. ALL FRAMING EXPOSED TO WEATHER OR DESIGNATED "P.T." IS TO BE PRESSURE TREATED SOUTHERN PINE NO.2 PER "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", NFPA. ALL WOOD MEMBERS SHALL BE MANUFACTURED TO COMPLY WITH PS20 OF "AMERICAN SOFTWOOD LUMBER STANDARDS". MINIMUM MEMBER PROPERTIES SHALL BE IN ACCORDANCE WITH TABLE 4B IN THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION".
- C. ALL EXTERIOR WALL STUDS ARE TO BE 2x6'S SPACED AT 16" O.C. (U.N.O.). PLACE DOUBLE STUDS AT END OF WALLS AND TRIPLE STUDS AT INTERSECTIONS AND CORNERS, ALL MULTIPLE STUD POSTS SHALL BE FASTENED AS FOLLOWS: DOUBLE STUDS SHALL BE NAILED TOGETHER WITH 10d AT 6" O.C. TRIPLE STUDS SHALL BE NAILED TOGETHER WITH 16d AT 6" O.C. EACH SIDE, FOR (4) STUD POSTS, USE 20d NAILS AT 8-INCHES ON CENTER.
- D. ROOF SHEATHING SHALL BE 5/8-INCH, CDX, APA STRUCTURAL I RATED SHEATHING, EXPOSURE I, PER THE "AMERICAN PLYWOOD ASSOCIATION." SHEATHING SHALL BE FASTENED WITH 8d NAILS AT 6-INCHES ON CENTER AT PANEL EDGES AND AT 12-INCHES ON CENTER AT ALL INTERMEDIATE SUPPORTS.
- WALL SHEATHING SHALL BE 1/2-INCH, CDX, APA STRUCTURAL I RATED SHEATHING, EXPOSURE I, PER THE "AMERICAN PLYWOOD ASSOCIATION." SHEATHING SHALL BE FASTENED WITH 8d NAILS AT 6-INCHES ON CENTER AT PANEL EDGES AND AT 12-INCHES ON CENTER AT ALL INTERMEDIATE
- F. ALL PLYWOOD SUBFLOORING SHALL BE 3/4-INCH THICK T&G, APA RATED 32/16 ADVANTECH SHEATHING OR STURD-I-FLOOR 20 OC RATED. SHEATHING SHALL BE GLUED WITH SUB-FLOOR ADHESIVE AND BE FASTENED WITH 8d NAILS AT 6-INCHES ON CENTER AT PANEL EDGES AND AT 12-INCHES ON CENTER AT ALL INTERMEDIATE SUPPORTS.
- G. LAMINATED VENEER LUMBER (L.V.L.) SHALL BE INSTALLED AND FASTENED PER THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM MEMBER PROPERTIES SHALL BE AS FOLLOWS:
- FLEXURE: Fb = 2,600 PSI
- 2. SHEAR: Fv = 285 PSI
- MODULUS OF ELASTICITY: E = 2,000,000 PSI
- CONTRACTOR SHALL PROVIDE MANUFACTURER'S PRODUCT SHEETS FOR APPROVAL BY ENGINEER FOR ALL LVL BEAMS
- H. 1½-INCH LAMINATED STRAND VENEER LUMBER (L.S.L.) SHALL BE INSTALLED AND FASTENED PER THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM MEMBER PROPERTIES SHALL BE AS FOLLOWS:
- FLEXURE: Fb = 1,700 PSI
- SHEAR: Fv = 425 PSI
- MODULUS OF ELASTICITY: E = 1,300,000 PSI CONTRACTOR SHALL PROVIDE MANUFACTURER'S PRODUCT SHEETS FOR
- APPROVAL BY ENGINEER FOR ALL LSL MEMBERS PROVIDE MIN. 3" BEARING FOR ALL LAMINATED VENEER AND PARALLEL
- STRAND BEAMS, 2" BEARING FOR STANDARD LUMBER BEAMS. J. ALL WOOD TOP PLATE SPLICES SHALL BE STAGGERED 6'-0" MINIMUM.

- K. ALL WALL SHEATHING SHALL BE CONTINUOUS BETWEEN TOP PLATES AND BOTTOM PLATE OF WALL ABOVE. ALL PLYWOOD PANELS EDGES SHALL BE CONTINUOUSLY BLOCKED AND NAILED.
- L. ALL MULTIPLE MEMBERS ARE TO BE FASTENED TOGETHER WITH THE FOLLOWING NAILS AND SIMPSON SDS (STRONG-DRIVE SCREWS), USING THE FASTENER-TO-FASTENER SPACING NOTED WITHIN EACH ROW OF FASTENERS. ALL FASTENERS SHALL BE INSTALLED IN THE QUANTITY OF ROWS SPECIFIED, IN A STAGGERED PATTERN:

PLIES	DEPTH	FASTENERS	SPACING	ROWS
(2)1-1/2"	9"-12"	10d NAILS	12" O.C.	2
(3)1-1/2"	9"-12"	16d NAILS	16" O.C.	2*
(2)1-3/4"	9"-12"	12d NAILS	16" O.C.	2
(2)1-3/4"	14"-20"	12d NAILS	12" O.C.	3
(3)1-3/4"	9"-12"	SDS1/4"x4-1/2"	12" O.C.	2*
(3)1-3/4"	14"-20"	SDS1/4"x4-1/2"	12" O.C.	3*
(4)1-3/4"	9"-12"	SDS1/4"x6"	12" O.C.	2*
(4)1-3/4"	14"-20"	SDS1/4"x6"	12" O.C.	3*
		UADRUPLE-PLY MEN HE NUMBER OF ROW		

- SIDE-TO-SIDE SPACING SHALL ALSO BE STAGGERED. M. PROVIDE SOLID BLOCKING BETWEEN JOISTS AND RAFTERS AT ALL BEARING
- N. ALL MISCELLANEOUS WOOD CONNECTIONS SHALL BE FASTENED PER 2012 IBC, TABLE 2304.9.1 "FASTENING SCHEDULE,"
- O. NAILS INDICATED IN THE DRAWINGS, DETAILS, AND NOTES SHALL BE DEFINED AS FOLLOWS: 8d=0.131"x2.5", 10d=0.148"x3", 16d=0.162"x3.5". SUBSTITUTIONS FOR THESE NAIL SIZES SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR APPROVAL.
- P. DOUBLE JOISTS SHALL BE LOCATED BENEATH ALL PARTITIONS WHEN THE LENGTH OF THE PARTITION EXCEEDS ONE HALF THE SPAN.
- Q. JOIST HANGERS SHALL BE SIZED ACCORDING TO THE FOLLOWING SCHEDULE (U.N.O.), EXCEPT AT SLOPED MEMBER CONDITIONS:

SUPPORTED	HANGER
MEMBER	
EX, 2 1/2"x7 1/2" (SLOPED)	HU36
(2)2x8	LUS28-2
11 7/8" TJI-110	IUS1.81/11.88
11 7/8" TJI-210	IUS2.06/11.88
(2) 1 ³ / ₄ "x11 ⁷ / ₈ " LVL	MGU3.63-SDS
(4) 1 ³ / ₄ "x11 ⁷ / ₈ " LVL	HGU7.25-SDS

SOME HANGERS MAY REQUIRE 16d NAILS - REFER TO THE SIMPSON STRONG-TIE CATALOG FOR REQUIREMENTS. CONTRACTOR SHALL PROVIDE MANUFACTURER'S CUT SHEETS FOR ALL HANGER SUBSTITUTIONS.

- R. PROVIDE SIMPSON H2.5A HURRICANE CLIPS FASTENED TO THE OUTSIDE FACE OF THE DOUBLE TOP PLATE AT ALL ROOF RAFTER BEARING POINTS.
- S. WOOD I-JOISTS SHALL HAVE THE FOLLOWING MINIMUM EI PROPERTIES (IN.2-LBS.):
- 11 7/8" TJI/110 = 238,000,000 11 7/8" TJI/210 = 283,000,000
- T. ALL ROOF SHEATHING SHALL BE LAID CONTINUOUSLY BETWEEN THE EDGES OF THE ROOF.

CONCRETE

- A. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 301, ACI 318 AND ACI 302.
- B. CEMENT SHALL COMPLY WITH ASTM C150, TYPE I OR II.
- C. REINFORCING STEEL SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60. ALL REINFORCEMENT SPLICES SHALL BE A MINIMUM OF 40 BAR DIAMETERS.
- D. CAST-IN-PLACE CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH F'c = 3000 PSI FOR FOOTINGS AND FOUNDATION WALLS. F'c = 3500 PSI FOR EXTERIOR EXPOSED SLABS/STEPS AND FOUNDATIONS WALLS.
- PROVIDE 6x6-W1.4xW1.4 W.W.F. IN ALL SLAB-ON-GRADE. ALL WIRE FABRIC SHALL CONFORM TO ASTM A185. ALL MESH EDGES SHALL LAP A MINIMUM OF TWO (2) SQUARES.
- F. CONCRETE SLUMP SHALL = 4" ± 1".

LENGTH = 40x BAR DIAMETER.

- G. MINIMUM CONCRETE COVER BETWEEN FACE OF REINFORCING BAR AND FACE OF CONCRETE SHALL BE AS FOLLOWS:
- CONCRETE CAST AGAINST EARTH = 3"
- FORMED CONCRETE EXPOSED TO WEATHER OR EARTH = 2" H. ALL SLABS AND FOUNDATION WALLS EXPOSED TO WEATHER SHALL HAVE A
- MINIMUM AIR ENTRAINMENT OF 6% + 1.5% PER ACI- 318 4.2.1. PROVIDE CORNER BARS AT ALL WALL INTERSECTIONS WITH SIZE AND
- SPACING TO MATCH HORIZONTAL WALL REINFORCEMENT. PROVIDE KEYED JOINTS BETWEEN ALL NON-MONOLITHIC INTERSECTING
- CONCRETE WALLS AND AT ALL CONCRETE JOINTS. K. GROUT SHALL BE NON-SHRINKABLE, NON-METALLIC CONFORMING TO ASTM
- C1107, AND SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5,000 PSI. PREGROUTING OF BASE PLATES SHALL NOT BE PERMITTED. PROVIDE AN 8-MIL VAPOR BARRIER OVER A 4-INCH LAYER OF GRAVEL
- BENEATH ALL SLAB-ON-GRADE.
- M. ALL VERTICAL WALL REINFORCEMENT INTERRUPTED BY WALL OPENINGS SHALL BE PLACED IMMEDIATELY ADJACENT TO EACH OF THE OPENINGS. N. PROVIDE DOWELS WITH STANDARD BAR HOOK IN FOOTING TO MATCH
- O. PROVIDE 1/2-INCH DIAMETER ANCHOR BOLTS AT A MAXIMUM OF 6'-0" ON CENTER AT ALL WOOD SILL PLATES. PROVIDE AT LEAST (2) ANCHOR BOLTS PER PLATE SECTION WITH ONE BOLT LOCATED AT NOT MORE THAN 12-INCHES FROM EACH END. BOLTS SHALL EMBED AT LEAST 7-INCHES INTO MASONRY OR CONCRETE. NUTS AND PLATE WASHERS (1/8"x2"x2") SHALL BE TIGHTENED ONTO EACH BOLT.

DIAMETER AND SPACING OF VERTICAL REINFORCEMENT. MINIMUM SPLICE

IV. STRUCTURAL STEEL

- ALL STRUCTURAL STEEL SHALL BE ASTM FABRICATED AND ERECTED IN ACCORDANCE WITH AISC "STEEL CONSTRUCTION MANUAL" WITH A MINIMUM YIELD STRENGTH AS FOLLOWS:
- W SHAPES: Fy = 50 ksi, PER ASTM A 992.
- PLATES, ANGLES AND CHANNELS: Fy = 36 ksi PER ASTM A36.
- B. ALL STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A53, GRADE B, Fy = 35 ksi. STANDARD SCHEDULE 40 PIPE TO BE USED UNLESS NOTED OTHERWISE.
- C. ALL BOLTS SHALL BE ASTM A307.
- D. WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE "STRUCTURAL WELDING CODE" AWS D1.1-2006. USE 70 KSI, LOW-HYDROGEN ELECTRODES.
- E. ALL STEEL LINTELS SHALL BE GALVANIZED AND HAVE A MINIMUM OF 6" BEARING AND SHALL BE PROPORTIONED AS FOLLOWS FOR EACH 4" OF WALL

וטוח.			
	OPENING SIZE	LINTEL	
	UP TO 4'-0"	L4x3 1/2x1/4	
	4'-1" TO 5'-0"	L4x3 1/2x5/16	
	5'-1" TO 6'-0"	L5x3 1/2x5/16	
	6'-1" TO 8'-0"	L6x3 1/2x3/8	

- CONTRACTOR SHALL DESIGN AND ERECT SHORING AND/OR BRACING OF EXISTING WALLS AS REQUIRED DURING INSTALLATION OF LINTELS. DESIGN AND ERECTION OF SHORING AND/OR BRACING SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- G. NO OPENINGS IN BEAMS OR COLUMNS ARE PERMITTED WITHOUT STRUCTURAL ENGINEER'S APPROVAL.
- SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE AND CONNECTION TO BE MADE.
- ALL MISCELLANEOUS STEEL CONNECTIONS SHALL BE WELDED ALL AROUND WITH ONE-QUARTER-INCH FILLET WELD UNLESS OTHERWISE NOTED, EXCEPT FOR SLOTTED CONNECTIONS.
- J. PROVIDE A MINIMUM BEARING LENGTH OF 6" FOR ALL BEAMS SUPPORTED ON
- K. HANDRAILS AND GUARDRAILS SHALL BE DESIGNED BY THE MANUFACTURER'S ENGINEER FOR THE MOST RESTRICTIVE OF THE LOADS GIVEN AND APPLICABLE DESIGN CODE. IN NO CASE SHALL TOTAL COMBINED POST/RAILING DEFLECTION EXCEED 0.50". SUBMIT SHOP DRAWINGS BEARING THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT TO THE ARCHITECT INDICATING ALL MEMBERS AND
- K. ALL WORK SHALL COMPLY WITH THE AISC CODE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" EXCEPT THAT PARAGRAPH 4.2.1 SHALL BE DELETED.

- ALL HOLLOW CONCRETE MASONRY UNITS SHALL BE LIGHT WEIGHT AND CONFORM TO ASTM C90 TYPE I HAVING A NET MINIMUM AREA COMPRESSIVE STRENGTH OF 1900 PSI AND A PRISM STRENGTH F'm = 2000 PSI IN ACCORDANCE WITH THE UNIT STRENGTH METHOD.
- ALL FACE BRICK MASONRY UNITS SHALL CONFORM TO ASTM C216 AND C652, GRADE MW OR SW, TYPE FBS, FBX, OR FBA. WITH A MINIMUM COMPRESSIVE STRENGTH = 2500 PSI.
- C. GALVANIZED HORIZONTAL JOINT REINFORCEMENT SHALL BE 9 GA, MINIMUM, PLACED IMMEDIATELY ABOVE AND BELOW ALL OPENINGS AND AT 16" O.C. VERTICALLY. REINFORCEMENT SHALL BE LADDER TYPE, AND WHERE SPLICED, SHALL LAP A MINIMUM OF 6". REINFORCEMENT SHALL CONFORM TO ASTM A-82 AND ASTM A153, CLASS B2, HOT DIP GALVANIZED (1.5 OZ./SF)
- D. ALL VERTICAL WALL REINFORCEMENT INTERRUPTED BY WALL OPENINGS SHALL BE PLACED IMMEDIATELY ADJACENT TO EACH OF THE OPENINGS.
- MASONRY MORTAR SHALL BE ASTM C270 TYPE S FOR HOLLOW CMU WALLS AND TYPE N FOR VENEER WALLS. PORTLAND CEMENT/LIME SHALL BE USED FOR ALL CMU WALLS.
- ALL MASONRY CELLS CONTAINING BOLTS OR REINFORCEMENT SHALL BE FILLED WITH COARSE GROUT PER ASTM C476, AGGREGATE PER ASTM C404.
- PROVIDE TWO (2) COURSES OF SOLID CMU PER ASTM C 90 OR GROUT-FILLED CMU BENEATH ALL BEAM, POSTS AND HEADER BEARING POINTS.
- H. PROVIDE DOWELS WITH STANDARD BAR HOOK IN FOOTING TO MATCH DIAMETER AND SPACING OF VERTICAL REINFORCEMENT. MINIMUM SPLICE LENGTH = 40x BAR DIAMETER.
- I. BRICK TIES SHALL BE ATTACHED TO ALL BRICK VENEER SPACED AT 24" O.C. HORIZONTALLY AND 16" O.C. VERTICALLY (MAXIMUM). CORRUGATED TIES ARE PROHIBITED FOR WALLS WITH CAVITIES OVER 1", TIES SHALL EXTEND 3" INTO BRICK AND/OR CMU.
- TIE MATERIAL SHALL CONFORM TO ASTM A366 AND ASTM A153, CLASS B2, HOT DIP GALVANIZED (1.5 OZ/SF.) STEEL WIRE SHALL CONFORM TO ASTM A82.
- K. ALL PRECAST MASONRY LINTELS SHALL HAVE A MINIMUM BEARING LENGTH OF 8" AND SHALL BE PROPORTIONED AS FOLLOWS FOR EACH 4" OF THICKNESS FOR NON-LOAD-BEARING WALLS, F'c=3000 PSI.

OPENING THICKNESS LINTEL (8" DEEP) UP TO 4'-0" W/1 #4 T&B 4'-1" TO 5'-0" W/1 #5 T&B 5'-1" TO 8'-0" W/1 #6 T&B

- ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE "SPECIFICATIONS FOR MASONRY STRUCTURES" ACI 530.1-02/ASCE 6-02/TMS
- M. ALL CMU GROUT SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH OF 2000
- N. PROVIDE CRACK CONTROL JOINTS AT ALL WALL RETURNS AND JAMBS OF OPENINGS AND AT A MAXIMUM SPACING OF 25-FEET ON CENTER.

GENERAL

- INFORMATION SHOWN REGARDING EXISTING CONDITIONS HAS BEEN OBTAINED BY LIMITED VISUAL OBSERVATIONS. AREAS NOT VISIBLE HAVE BEEN ASSUMED TYPICAL WITH OBSERVED EXISTING CONDITIONS.
- B. THE CONTRACTOR SHALL EXPOSE AND CONFIRM ALL EXISTING STRUCTURAL CONDITIONS RELATIVE TO THE NEW CONSTRUCTION AND INFORM THE ENGINEER OF CONDITIONS AT VARIANCE WITH THOSE SHOWN ON THE DRAWINGS. VERIFICATION AND NOTIFICATION SHALL PROCEED PRIOR TO

- THE START OF WORK SO THAT ANY NECESSARY CHANGES CAN BE MADE WITHOUT DELAYING THE PROJECT SCHEDULE.
- C. THE CONTRACTOR SHALL MEASURE AND PROVIDE ALL EXISTING FIELD DIMENSIONS, ELEVATIONS AND CONDITIONS AT THE JOB SITE PRIOR TO CONSTRUCTION AND THE SUBMISSION OF SHOP DRAWINGS, AND SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.
- D. THE CONTRACTOR SHALL RETAIN THE SERVICES OF A DISTRICT OF COLUMBIA REGISTERED ENGINEER TO DESIGN ALL TEMPORARY BRACING AND SHORING, AS NEEDED, TO ENSURE VERTICAL AND LATERAL STABILITY OF THE ENTIRE STRUCTURE OR PORTION THEREOF DURING DEMOLITION
- E. THE CONTRACTOR'S REGISTERED ENGINEER SHALL ALSO DETERMINE THE ALLOWABLE TEMPORARY CONSTRUCTION LOADS & PROVIDE A DESIGN FOR ALL FALSEWORK, FORMWORK, STAGINGS, RIGGING, BRACING, SHEETING, & SHORING, AND TEMPOARY CONSTRUCTION EQUIPMENT SUCH AS MANLIFTS AND PALLET TRUCKS WHENEVER THE LOADING FROM THE CONTRACTOR'S WORK EXCEEDS THE ALLOWABLE LIVE LOAD CAPACITIES INDICATED ON THE DRAWINGS. THE DESIGN PROCEDURES SHALL CONFORM TO ALL GOVERNING CODES & SAFETY REQUIREMENTS, A RECORD COPY OF THE SIGNED & SEALED DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE A/E.
- F. DETAILS, SECTIONS, AND NOTES SHOWN ON THESE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR CONDITIONS ELSEWHERE UNLESS OTHERWISE SHOWN OR NOTED.
- G. ALL WALLS ARE DESIGNED AS LATERALLY BRACED BY THE FLOOR AND ROOF SYSTEMS. CONTRACTOR SHALL ENSURE THAT WALLS ARE ADEQUATELY BRACED DURING CONSTRUCTION.

H. TEMPORARY BRACING SHALL BE PROVIDED FOR ALL WALLS SUBJECT TO

UNBALANCED BACKFILL. BRACE WALL PLUMB UNTIL STABILIZING ELEMENT AT TOP AND BOTTOM OF WALL IS IN PLACE. I. ANY REQUIRED TEMPORARY SHORING SHALL BE IN CONFORMANCE WITH

OSHA REGULATIONS, UNBRACED EXCAVATIONS SHALL BE SLOPED NO

- GREATER THAN (1.5) HORIZONTAL TO (1) VERTICAL. J. CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN VICINITY OF FOUNDATIONS AND NOTIFY THE ARCHITECT IF A CONFLICT EXISTS. PROVIDE INFORMATION ON LOCATION SIZE AND ELEVATION OF UTILITIES PRIOR TO START OF WORK SO THAT ANY NECESSARY CHANGES CAN BE MADE
- WITHOUT DELAYING THE PROJECT SCHEDULE. K. THE DEVELOPMENT AND IMPLEMENTATION OF JOB SITE SAFETY AND CONSTRUCTION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE
- L. CONTRACTOR SHALL PROVIDE INDEPENDENTLY PREPARED SHOP DRAWINGS AND SHALL NOT REPRODUCE ANY PORTION OF THE CONTRACT DOCUMENTS IN PREPARING SHOP DRAWINGS. THE SHOP DRAWINGS SHALL NOT SIMPLY BE A MARK-UP OF THE CONTRACT DOCUMENTS.

A. ALL MEANS AND METHODS OF SAFELY REMOVING ALL EXISTING CONSTRUCTION SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

VIII. TESTING AND INSPECTION

NOTED OTHERWISE.

INFILTRATION.

GENERAL CONTRACTOR.

- THE CONTRACTOR SHALL RETAIN THE SERVICES OF AN INSPECTION AGENCY TO PERFORM THE FOLLOWING SERVICES.
- A. INSPECTION OF SUBGRADE BELOW ALL FOUNDATIONS AND SLAB-ON-GRADE TO VERIFY THE ADEQUACY OF THE BEARING MATERIAL. B. WRITTEN REPORTS SHALL BE SUBMITTED TO THE ARCHITECT STATING COMPLIANCE OR NONCOMPLIANCE WITH DESIGN DOCUMENTS AND

SPECIFICATIONS. ALL REPORTS SHALL BE SIGNED AND SEALED BY DISTRICT

OF COLUMBIA REGISTERED ENGINEER. C. INSPECTION AND TESTING OF ALL NEW STRUCTURAL FILL WITH REPORTS SUBMITTED TO ARCHITECT STATING COMPLIANCE OR NONCOMPLIANCE WITH

PERCENT COMPACTION REQUIREMENTS.

IX. EARTHWORK

- A. ALLOWABLE SOIL BEARING PRESSURE FOR ALL SHALLOW FOOTINGS IS TO BE 2000 PSF PER THE GEOTECHNICAL REPORT PREPARED BY FMC & ASSOCIATES, LLC, DATED FEBRUARY 6, 2015. SHOULD UNSUITABLE MATERIAL BE ENCOUNTERED, FOOTINGS SHALL BE OVEREXCAVATED AND REPLACED WITH LEAN CONCRETE, F'c = 2000 PSI. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW EXTERIOR GRADE, UNLESS
- B. ALL FILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL AND SHALL BE SELECTED ON THE BASIS OF LABORATORY COMPACTION TESTS, HAVING A LIQUID LIMIT OF LESS THAN 40, A PLASTICITY INDEX OF LESS THAN 16, FILL SHALL BE PLACED IN MAXIMUM 8-INCH LIFTS AND COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY OBTAINED BY ASTM D1557, MODIFIED PROCTOR METHOD.
- C. IF HIGH PLASTICITY SOILS ARE ENCOUNTERED AT OR NEAR FOOTING SUBGRADE DURING CONSTRUCTION, THE MATERIAL SHALL BE REMOVED AND REPLACED WITH PROPERTY COMPACTED STRUCTURAL FILL.
- D. FOOTING TRENCHES SHALL BE BACKFILLED WITH LEAN CONCRETE IMMEDIATELY UPON EXCAVATION TO PREVENT GROUNDWATER
- E. PERIMETER DRAIN TILE SHALL CONSIST OF 4-INCH DIAMETER CORRUGATED POLYETHYLENE TUBING PER ASTM D-405 WITH A MAXIMUM SIZE WIDTH OF 1/4-INCH. TUBING SHALL BE PLACED WITH SLOTS DOWN USING STRAIGHT SECTIONS AND STANDARD CONNECTIONS.

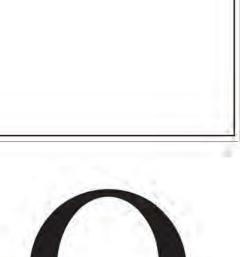
X. STEEL STAIRS

- A. ALL STEEL STAIR STRINGERS SHALL BE CONTINUOUS HSS5x4x³ MINIMUM, UNLESS OTHERWISE NOTED, STRINGERS SHALL BE MITERED AND WELDED WITH FULL PENETRATION WELDS.
- B. ALL RISER PLATES SHALL BE !" STEEL PLATES, MINIMUM.

FOR SLOTTED CONNECTIONS.

APPROVAL.

- C. ALL TREAD PLATES SHALL BE 3" STEEL PLATES, MINIMUM AND SHALL BE FASTENED TO THE 3" WOOD STAIR TREADS WITH TWO ROWS OF #12 WOOD
- SCREWS AT SIX INCHES ON CENTER. D. ALL MISCELLANEOUS STEEL CONNECTIONS SHALL BE WELDED ALL AROUND WITH ONE-QUARTER-INCH FILLET WELD UNLESS OTHERWISE NOTED, EXCEPT
- E. ALL STEEL STAIRS ARE TO BE ENGINEERED BY THE CONTRACTOR'S FABRICATOR FOR THE LOADS GIVEN AND ALL APPLICABLE CODES WITH SHOP DRAWINGS STAMPED BY THE MANUFACTURER'S REGISTERED ENGINEER LINCENSED IN WASHINGTON, DC AND SUBMITTED TO THE ARCHITECT FOR



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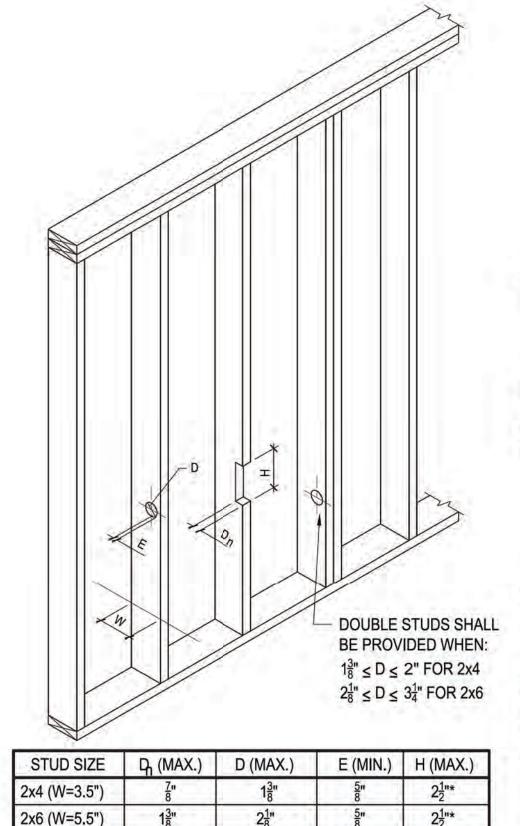
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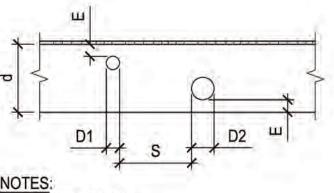
WASHINGTON, DC 20007

LOT: 0046 SQUARE: 1209

31ST ST NW

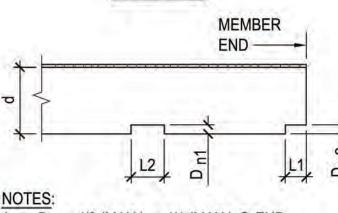
DESIGN NOTES





1. D = d/3 (MAX.) 2. E = 2" (MIN.) 3. S = 2" (MIN.)

BORED HOLE



- D_{nx}= d/6 (MAX.), = d/4 (MAX.) @ END
- 2. Lx = d/3 (MAX.) NO NOTCHES PERMITTED IN CENTER 1/3 OF SPAN.

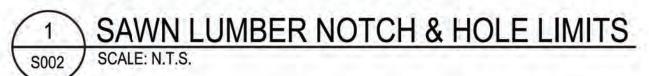
EDGE NOTCH

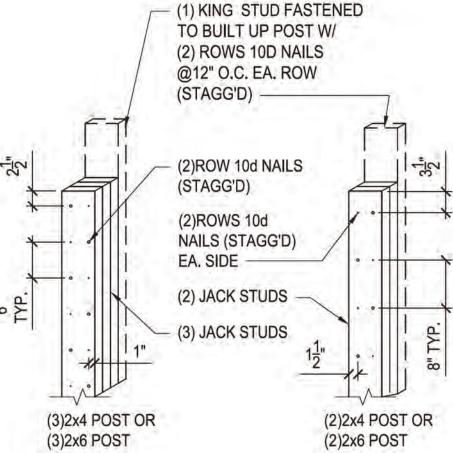
SOLID JOISTS, RAFTERS, & BEAMS

LOAD-BEARING STUD WALLS

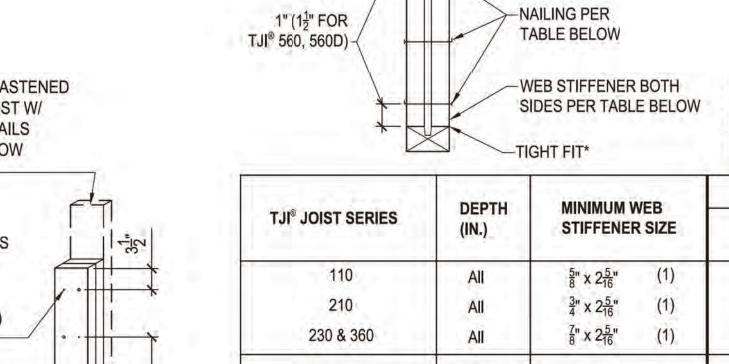
THESE LIMITS ARE IN GENERAL ACCORDANCE WITH THE 2012 IRC. NOTCHES OR HOLES THAT DO NOT ARE CUT, OR WHEN CUT MEMBERS SUPPORT MORE THAN ONE LEVEL, MUST BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW.

- ALL BORED HOLES WITH AN EDGE DISTANCE LESS THAN THE DIMENSION "E" NOTED ABOVE SHALL BE REINFORCED WITH SIMPSON "SS" STUD SHOES (OR APPROVED EQUAL). WHERE BORED HOLES PROVIDE PASSAGE FOR PIPING, SIMPSON NS2 (OR APPROVED EQUAL) SHALL BE PROVIDED FOR CODE-REQUIRED
- * EDGE NOTCH HEIGHT, "H", IS LIMITED BY THE METAL STUD SHOE DIMENSION PER SIMPSON.
- GENERAL NOTATION: D = DIAMETER, OF BORED HOLE Dnx = NOTCH DEPTH, H = NOTCH HEIGHT, Lx = NOTCH LENGTH, E = EDGE DISTANCE, d = JOIST DEPTH, W = STUD DEPTH





TYP. NAILING SCHEDULE FOR 2x BUILT UP POSTS SCALE: 3/4"=1'-0"



NUMBER NAILS TYPE END INTERMEDIATE $(0.113" \times 2\frac{1}{2}")$ 560 2x4 (2) All $(0.135" \times 3\frac{1}{2}")$ 18" 16d 20" $(0.135" \times 3\frac{1}{2}")$ 22" (3) 2x4 (2) 11 13

* WITH POINT LOAD FROM ABOVE, AND NO

NAILING REQUIREMENTS

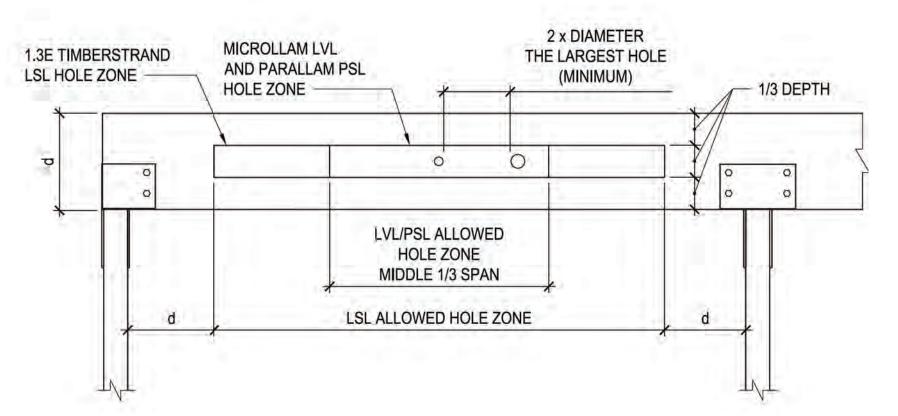
(1) PS1 OR PS2 SHEATHING, FACE GRAIN VERTICAL

(2) CONSTRUCTION GRADE OR BETTER

(3) WEB STIFFENERS ARE ALWAYS REQUIRED FOR 22" AND 24" TJI® 560D JOISTS

23" MAXIMUM

WEB STIFFENER ATTACHMENT REQUIREMENTS FOR TJI'S SCALE: 1/2"=1'-0" S002

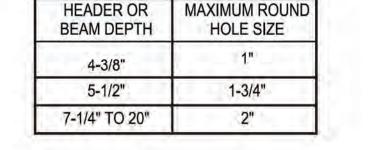


1. ALLOWED HOLE ZONE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADS ONLY.

2. ROUND HOLES ONLY

3. NO HOLES IN CANTILEVERS.

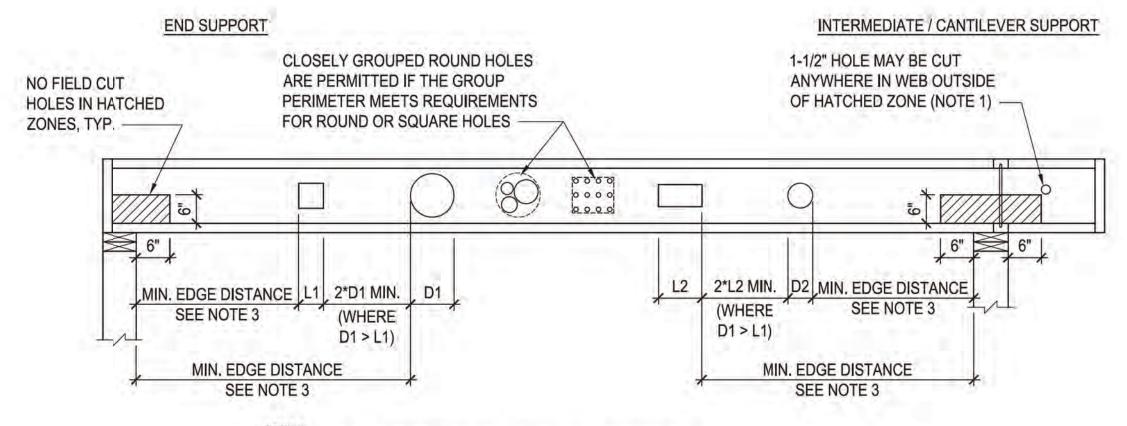
- 4. NO HOLES IN HEADERS OR BEAMS IN PLANK ORIENTATION
- 5. ALLOWABLE PENETRATIONS SUITABLE ONLY FOR WEYERHAEUSER LVL, LSL AND PSL PRODUCTS.
- 6. CONTRACTOR TO SUPPLY ALLOWABLE PENETRATION CUTSHEET FOR SUBSTITUTIONS



DO NOT CUT, NOTCH OR DRILL HOLES IN HEADERS

AND TABLES.

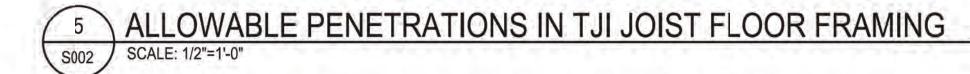
OR BEAMS EXCEPT AS INDICATED IN ILLUSTRATIONS

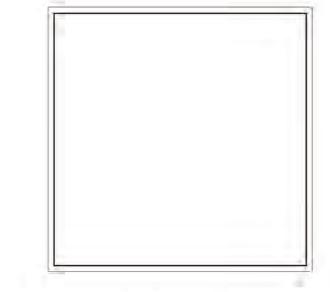


1. DO NOT CUT HOLES LARGER THAN 1-1/2" IN CANTILEVER

2. ALLOWABLE PENETRATIONS SUITABLE ONLY FOR WEYERHAEUSER TJI PRODUCTS.

- CONTRACTOR TO SUPPLY ALLOWABLE PENETRATION CUTSHEET FOR SUBSTITUTIONS.
- SEE MANUF. DESIGN GUIDE FOR MINIMUM EDGE DISTANCE 4. DO NOT CUT TOP OR BOTTOM FLANGE.
- L1 MAX. WIDTH RECTANGULAR HOLE @ END SUPPORT;
- D1 MAX. DIAMETER HOLE @ END SUPPORT;
- L2 MAX. WIDTH RECTANGULAR HOLE @ INTERMEDIATE/CANTILEVER SUPPORT; D2 MAX. DIAMETER HOLE @ INTERMEDIATE SUPPORT. SEE MANUF. DESIGN GUIDE FOR DIMENSIONS SPECIFIC TO JOIST DESIGNATION.





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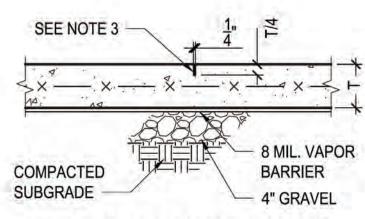
LOT: 0046 SQUARE: 1209

SECTIONS AND DETAILS

S002

DATE: 03-11-2021

ALLOWABLE PENETRATIONS IN LVL BEAMS S002 / SCALE: 1/2"=1'-0"



FOR SLAB THICKNESS "T", SEE PLAN

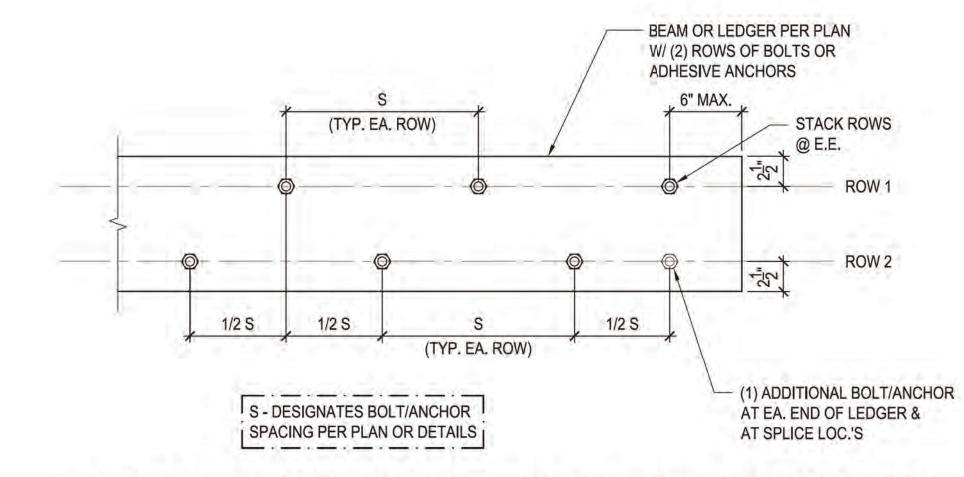
CONTROL JOINT

SCALE: 1"=1'-0"

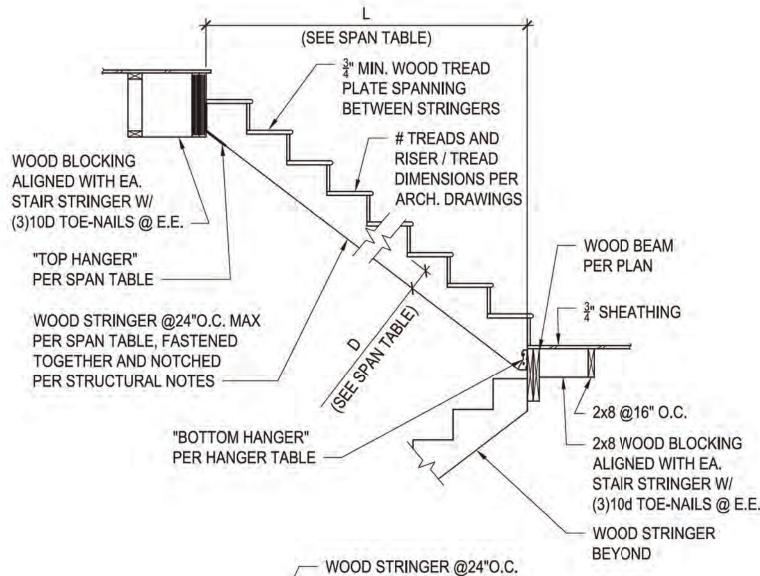
S003

- PROVIDE SUPPORT TO HOLD REINF. IN POSITION. SAW CUT CONTROL JOINTS TO DIVIDE SLAB INTO NEAR-SQUARE SHAPES NOT EXCEEDING 400 SQ. FT. IN AREA WHEN CONC. HAS HARDENED SUFFICIENTLY TO PERMIT CUTTING WITHOUT CHIPPING, SPALLING, OR TEARING.
- 3. FILL JOINT W/ EPOXY SEALANT AFTER SLAB HAS CURED.

TYP. S.O.G. CONTROL JOINT DET.



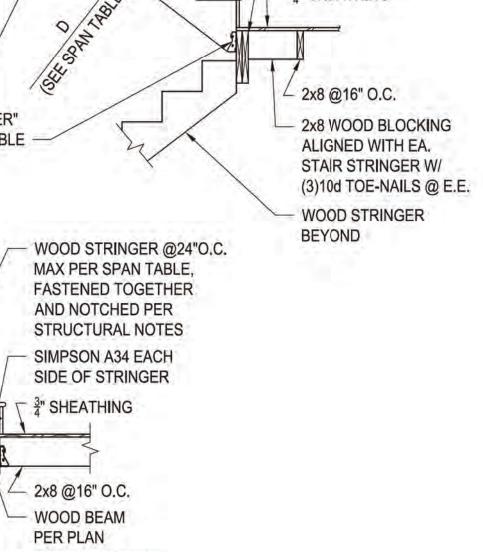
TYP. BEAM/LEDGER STAGGERED BOLT SPACING DETAIL S003 SCALE: 1 1/2"=1'-0"

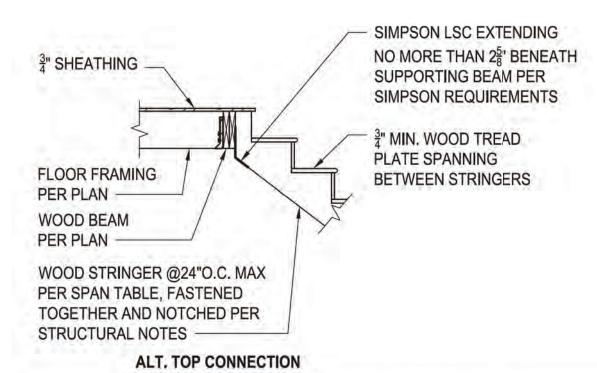


WOOD STRINGER

BEYOND

ALT. BOTTOM CONNECTION





	STAIR STRINGER SPAN TABLE						
MAX. SPAN (L)	MIN. DEPTH (D)	MIDDLE STRINGER	EDGE STRINGER	TOP HANGER	BOTTOM HANGER (MIDDLE)	BOTTOM HANGER (EDGE)	
7'-0"	5 <u>1</u> "	2x12	2x12	LSC	LUS26	LUC26	
9'-0"	5 1 "	(2)2x12	(2)2x12	(2)LSC	LUS26-2	HUC26-2	
10'-6"	5 <u>1</u> "	(3)2x12	(2)2x12	(2)LSC	LUS26-3	HUC26-2	
11'-6"	5 3 "	(2)1 ³ / ₄ "x11 ⁷ / ₈ " LVL	(2)1 ³ / ₄ "x11 ⁷ / ₈ " LVL	(2)LSC	HU46	HUC46	
13'-0"	5 3 "	(3)1 ³ / ₄ "x11 ⁷ / ₈ " LVL	(2)1 ³ / ₄ "x11 ⁷ / ₈ " LVL	(2)LSC	HU66	HUC46	
15'-6"	73"	(2)1 ³ / ₄ "x14" LVL	(2)1 ³ / ₄ "x14" LVL	(2)LSC	HU48	HUC48	
17'-6"	73"	(3)1 ³ / ₄ "x14" LVL	(2)1 ³ / ₄ "x14" LVL	(2)LSC	HU68	HUC48	

- 1. PRE-DRILL NOTCH CORNERS WITH A 4"Ø HOLE TO REDUCE STRESS CONCENTRATION AND DO NOT OVER-CUT NOTCHES.
- 2. DO NOT EXCEED 24" O.C. STRINGER SPACING.
- 3. HANGERS BY SIMPSON OR APPROVED EQUIVALENT.
- 4. USE MAX. NAILING U.N.O. AS SPECIFIED BY SIMPSON.





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FOR

1231-35 31ST ST NW

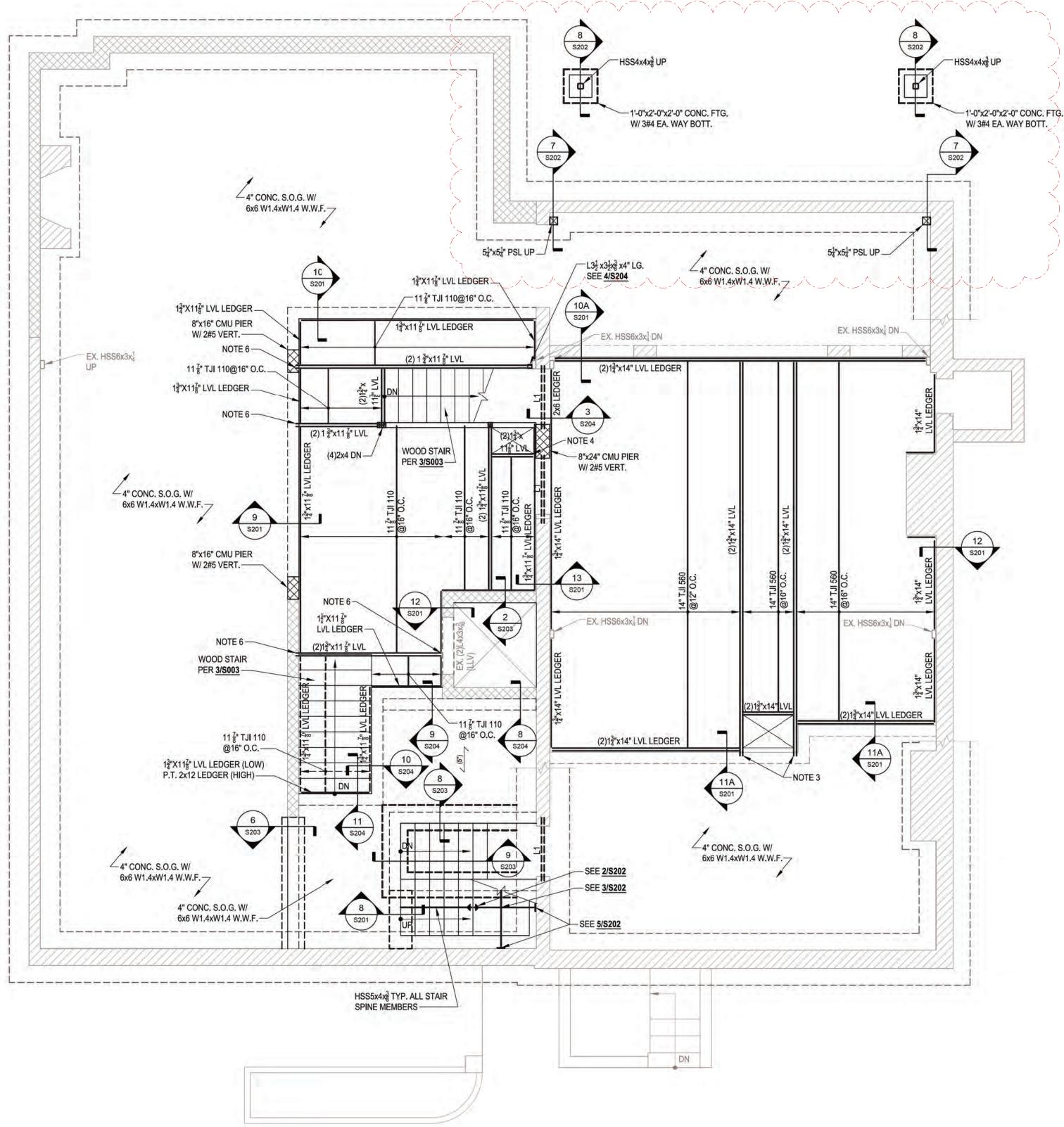
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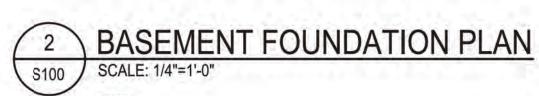
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SECTIONS AND DETAILS

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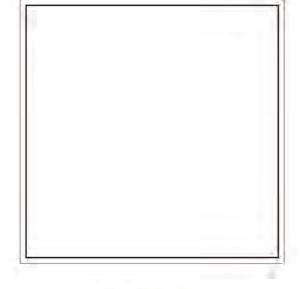
NOTES:

- ALL POSTS SHOWN ARE (2)2x6 DN IN 2x6 WALLS AND (2)2x4 DN IN 2x4 WALLS U.N.O.
- EXISTING CONDITIONS SHOWN ARE ASSUMED AND SHALL BE VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR.
- VERIFIED IN THE FIELD BY THE GENERAL CONTRACTO
- POCKET BEAM 4" INTO WALL PER 1/S202.

 A PROVIDE (A) ADDITIONAL ANCHORS IN LEG.
- PROVIDE (4) ADDITIONAL ANCHORS IN LEDGER AT HEADER CONNECTION LOCATION (2 EA. SIDE).
- 5. L1 DESIGNATES A GALVANIZED STEEL ANGLE LINTEL PER THE
- STRUCTURAL DESIGN NOTES.

 6. DISCONTINUE LEDGER AND CONNECT LVL BEAM DIRECTLY TO CMU
- OR CONC. WALL W/ SIMPSON HU412 HANGER W/ TITEN 2 SCREWS.

 7. (5") DESIGNATES THE SPAN DIRECTION OF A 5" CONC. SLAB W/ #4@12" O.C. BOTT-MOST IN THE SPAN DIRECTION AND #4@18" O.C. PERPENDICULAR TO THE SPAN DIRECTION.





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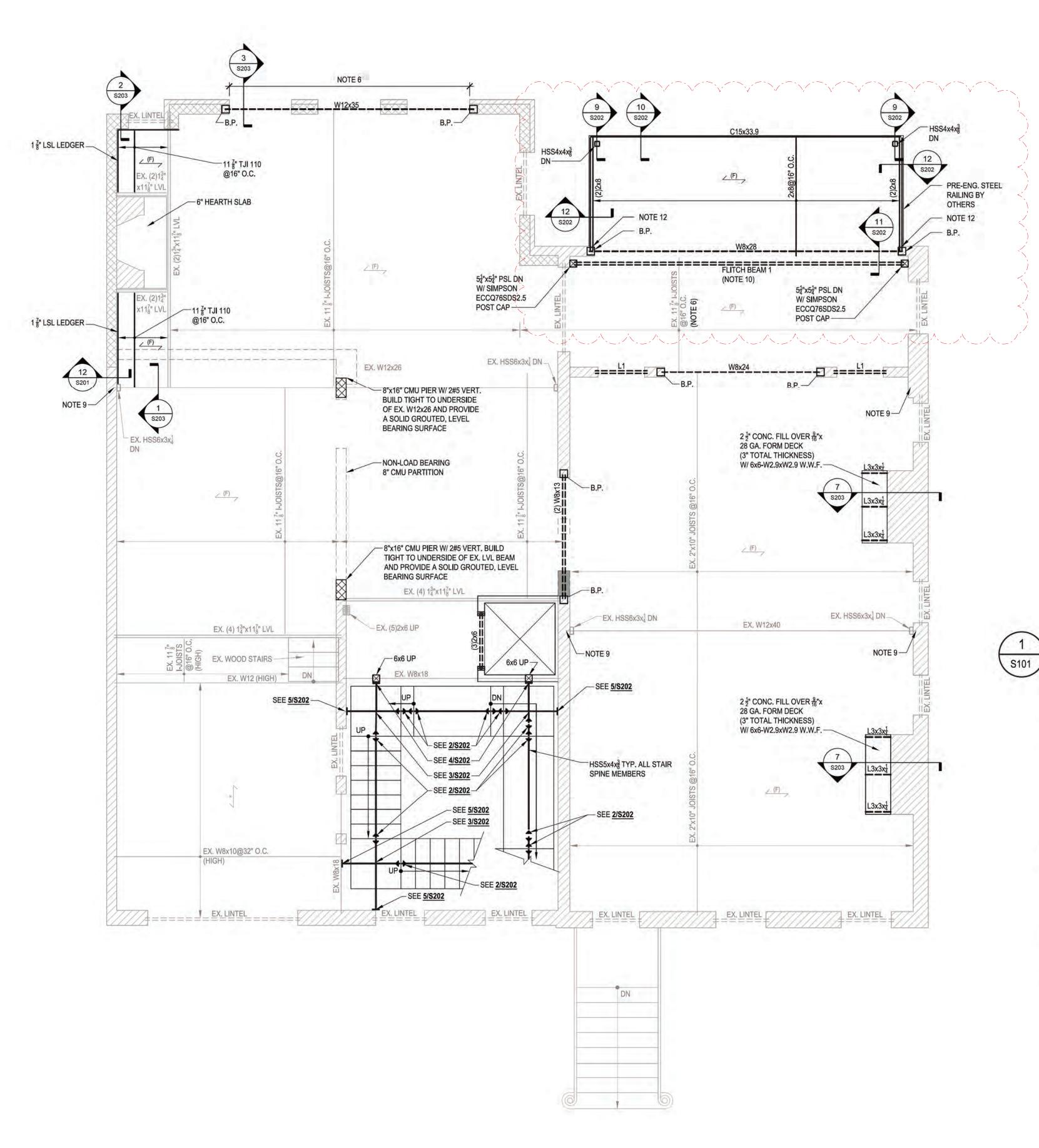
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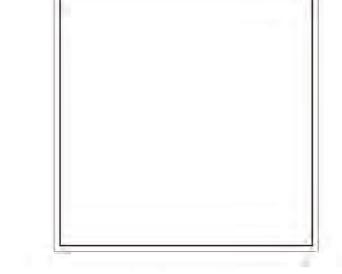
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BASEMENT FOUNDATION PLANS

S100

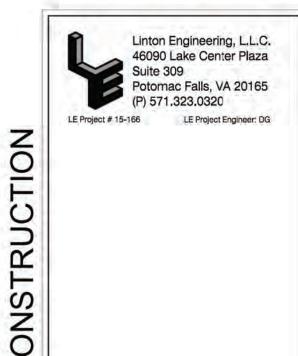






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FOR

FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-0"

NOTES:

- 1. ALL POSTS SHOWN ARE (2)2x6 UP IN 2x6 WALLS AND (2)2x4 UP IN 2x4 WALLS U.N.O.
- PROVIDE WOOD BEAM/JOIST HANGERS PER THE STRUCTURAL DESIGN NOTES.
- (F) DESIGNATES THE SPAN DIRECTION OF 3/4" SUBFLOOR
 PER THE STRUCTURAL DESIGN NOTES.
- DESIGNATES THE SPAN DIRECTION OF EXISTING FLOOR SHEATHING.
- DESIGNATES THE SPAN DIRECTION OF EXISTING CONCRETE SLAB ON METAL DECK.
- 4. DASHED LINES INDICATE DROPPED BEAMS/HEADERS, SOLID LINES
- INDICATE FLUSH FRAMED BEAMS/HEADERS.

 5. EXISTING CONDITIONS SHOWN ARE ASSUMED AND SHALL BE
- VERIFIED IN THE FIELD BY THE GENERAL CONTRACTOR.

 6. PROVIDE TEMPORARY SHORING OF EXISTING STRUCTURE AS NEEDED FOR NEW CONSTRUCTION AS DESIGNATED ON PLAN.

TRIM JOIST ENDS AS NEEDED FOR FIT-UP.

- 7. L1 DESIGNATES A GALVANIZED STEEL ANGLE LINTEL PER THE STRUCTURAL DESIGN NOTES.
- 8. B.P. DESIGNATES A STEEL BEARING PLATE PER <u>6/S202</u>.
- 4" MIN. BEARING (V.I.F.). FILL VOIDS AND PROVIDE A SOLID GROUTED LEVEL SURFACE 8" MIN. BENEATH BEAM BEARING POINT.
- 10. FLITCH BEAM 1 DESIGNATES (3) 1 3/2 x9 1/2 LVL & (2) 3/2 x9" STL. PL.
- 11. POCKET BEAM 4" INTO EX, MASONRY WALL PER

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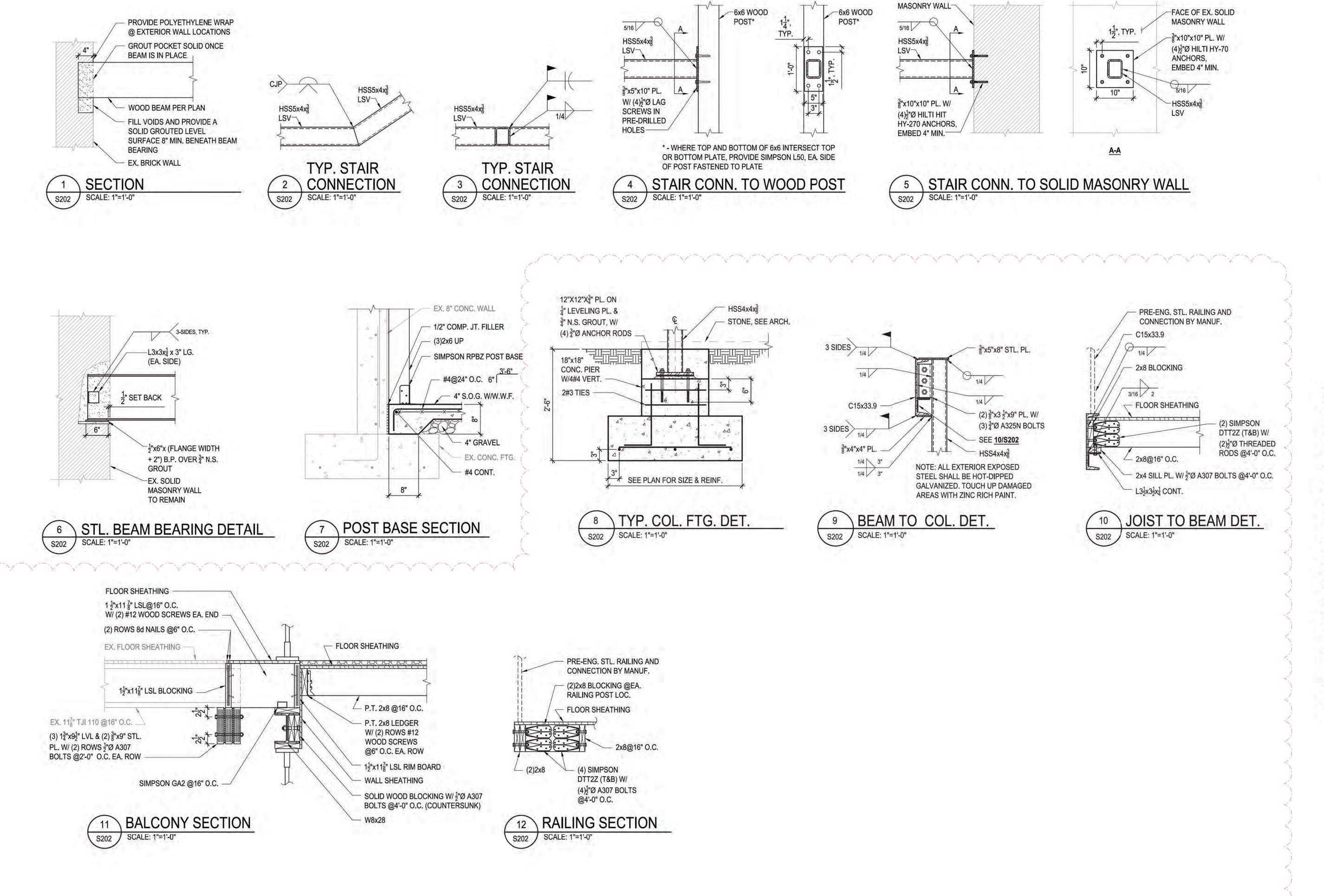
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FIRST FLOOR FRAMING PLAN

S101



EX. SOLID

Overmyer ARCHITECTS

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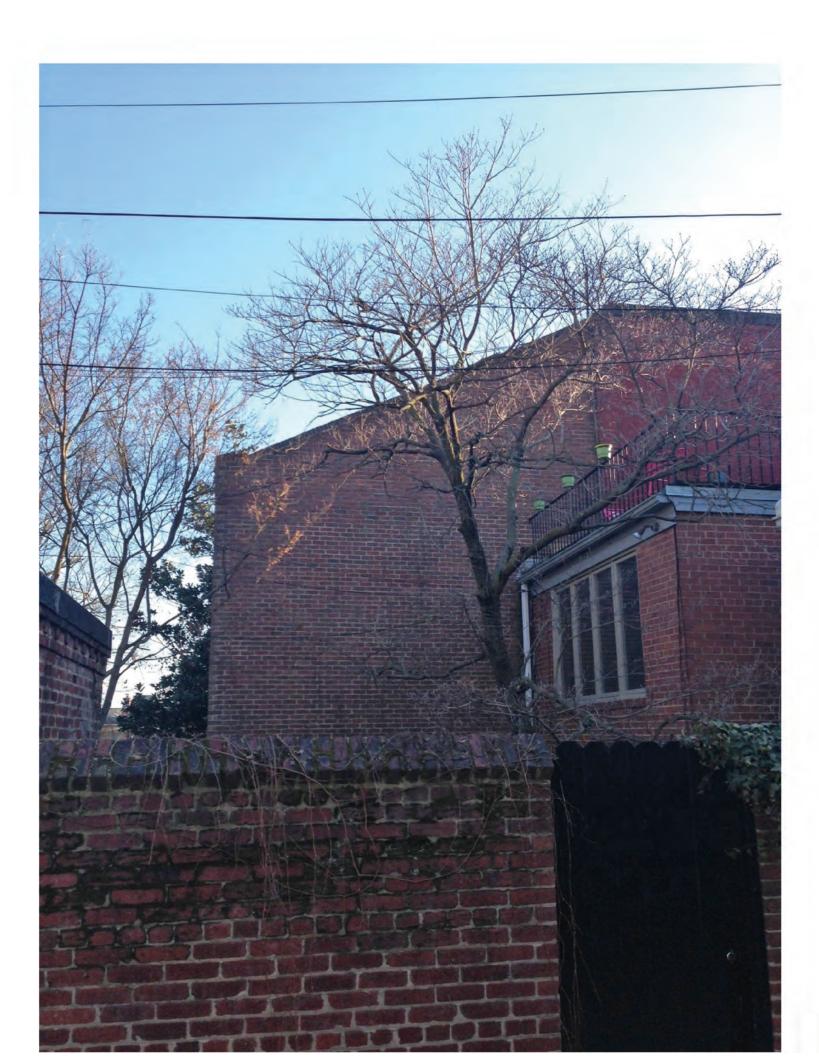
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SECTIONS AND DETAILS

S202



EAST ELEVATION



VIEW FROM PUBLIC ALLEY



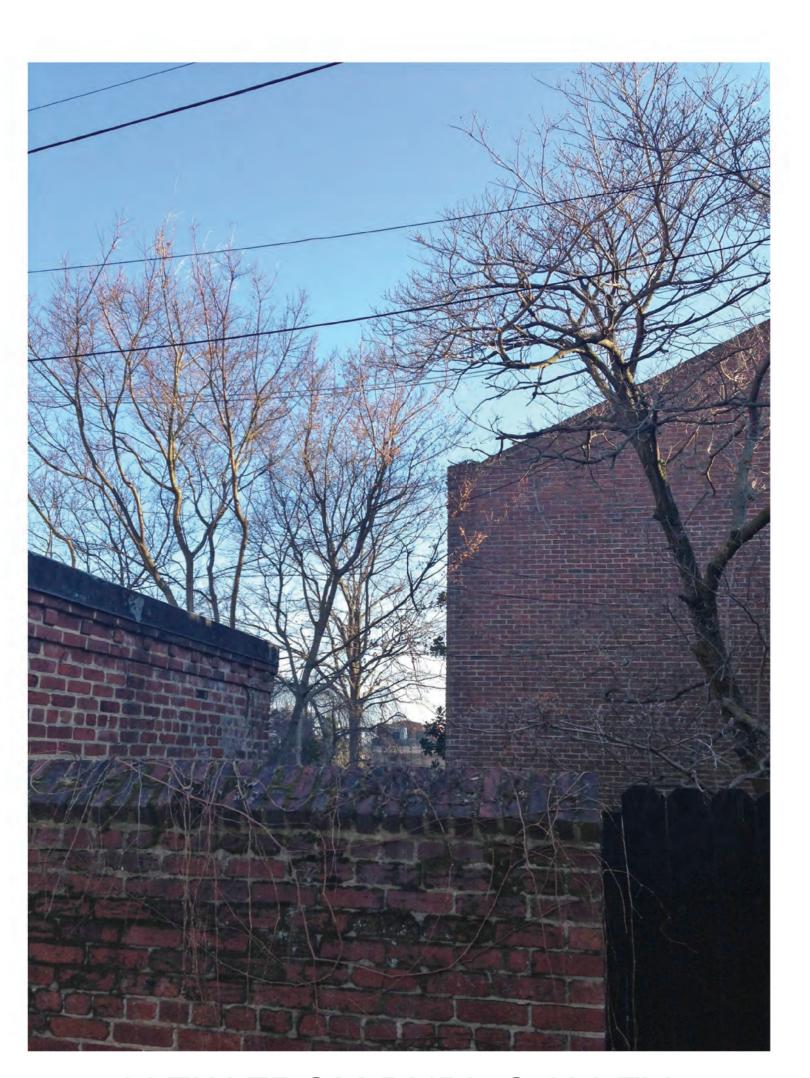
VIEW FROM OLD STONE HOUSE



VIEW FROM PUBLIC ALLEY



VIEW FROM OLD STONE HOUSE



VIEW FROM PUBLIC ALLEY



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PHOTOS

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